

**Air Preheater Pond
Location Restrictions Demonstration**

**W.A. Parish Electric Generating Station
Thompsons, Texas**

October 2018

Prepared For

NRG Texas Power LLC



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Certification

I, the undersigned Texas Professional Engineer, hereby certify that I am familiar with the technical requirements of Title 40 Code of Federal Regulations Part 257 Subpart D (§257). I also certify that it is my professional opinion that, to the best of my knowledge, information, and belief, that the information in this demonstration is in accordance with current good and accepted engineering practice(s) and standard(s), and meets the requirements of §257.60 through §257.64.

For the purpose of this document, "certify" and "certification" shall be interpreted and construed to be a "statement of professional opinion". The certification is understood and intended to be an expression of my professional opinion as a Texas Licensed Professional Engineer, based upon knowledge, information, and belief. The statement(s) of professional opinion are not and shall not be interpreted or construed to be a guarantee or a warranty of the analysis herein.

Jason Leik
Jason Leik, P.E.

91043
Texas License Number

[Signature]
Signature of Professional Engineer

10/12/18
Date



Section 1

Background

The purpose of this document is to demonstrate the compliance of the existing Air Preheater Pond (APH Pond) impoundment at the W.A. Parish Electric Generating Station (Station) with the location restrictions outlined in the Environmental Protection Agency's (EPA's) final coal combustion residuals (CCR) rule (Title 40 Code of Federal Regulations Parts 257 and 261) Subpart D - "Standards for the Disposal of Coal Combustion Residuals in Landfills and Surface Impoundments" (§257.60 through §257.64, federal rule). The E Pond is considered a CCR surface impoundment according to the federal rule (§257.53). This document includes information from a desktop study and Site visit to demonstrate that the E Pond is in compliance with placement above the uppermost aquifer (§257.60) and location with respect to wetlands (§257.61), fault areas (§257.62), seismic impact zones (§257.63), and unstable areas (§257.64).

1.1 Site Setting

The NRG Texas Power LLC (NRG) Station is located in Thompsons, Fort Bend County, Texas, adjacent to Smithers Lake (Figure 1). The electricity generating portion of the Site, or the main Plant Operations Area (Plant Area) is located along the southeastern shore of the lake.

According to the Geologic Atlas of Texas, Houston Sheet (BEG 1982), the Site is underlain by alluvium and the Beaumont Formation (also commonly referred to as Beaumont Clay). The alluvium is present at the SWDA CCR units and along the Brazos River, which is located approximately 0.9 miles from the northern boundary of the SWDA CCR units. The alluvium is not present at the Plant Area (APH Pond), which is consistent with this area being located outside of the Brazos River floodplain zone (FBC 2018). Both the alluvium and the Beaumont Formation are comprised of clay, silt, and sand, and may include stream channel, point bar, natural levee, backswamp, coastal marsh and mud flat deposits. The thickness of the Beaumont Formation is approximately 100 feet.

The alluvium and Beaumont Formation are located within the upper unit of the Chicot aquifer system. At most locations throughout Fort Bend County, the Chicot aquifer system is under confined conditions (TWDB 1990). The Chicot aquifer system is primarily recharged by precipitation at locations where it outcrops in Austin, Harris, and Waller Counties; groundwater then flows laterally within Fort Bend County (TWDB 1990).

1.2 Existing and Future Conditions

Some of the units managing CCR at the Station are subject to the EPA's final rule for the regulation and management of CCR under the Resource Conservation and Recovery Act (RCRA) Title 40, Code of Federal Regulations, Part 257 (40 CFR §257) (CCR rule, effective date October 17, 2015) and the CCR Remand Rule Proposal (March 1, 2018). CCR generated at the Station consist of fly ash, bottom ash, and FGD scrubber sludge, which have been classified by the Texas Commission on Environmental Quality (TCEQ) as Class II Nonhazardous. The Site has three active CCR management units that are subject to regulation under the CCR Rule and the CCR Remand Rule Proposal, as follows:

- Solid Waste Disposal Area (SWMU 001), which consists of active CCR management areas Cell 1C, Cell 2A, Cell 2B, and Cell 3 (the inactive cells at the SWDA are not subject to the CCR Rule);
- Air Preheater Pond (APH Pond, SWMU 021); and
- FGD Emergency Pond (E Pond, SWMU 020).

The APH Pond is located in the southwestern portion of the Plant Area as shown on Figure 2. According to NRG, the APH Pond comprises an area of 1.2 acres and has a total storage capacity of 3.7 acre-feet. The APH Pond is certified to be lined with a minimum of two feet of compacted soil with a hydraulic conductivity of no more than 1×10^{-7} centimeters per second (cm/sec) (Sargent & Lundy 2016). The APH Pond receives effluent from air preheater wash and boiler cleaning wash, which consists of fly ash or economizer ash particles and water.

Section 2

Location Restrictions

The location restrictions designated in the federal CCR rule are presented below with a corresponding demonstration to show compliance with each restriction. The location restrictions include placement above the uppermost aquifer, wetlands, fault areas, seismic impact zones, and unstable areas. Supporting information for the demonstrations is included in the appendices.

2.1 §257.60 – Placement above the Uppermost Aquifer

The federal CCR rule requires that CCR units such as the APH Pond must be constructed with a base that is located no less than 1.52 meters (five feet) above the upper limit of the uppermost aquifer, or must demonstrate there will not be an intermittent, recurring, or sustained hydraulic connection between any portion of the base of the CCR unit and the uppermost aquifer due to normal fluctuations in groundwater elevations (including the seasonal high water table).

Based on a review of Site-specific data, the uppermost *usable* aquifer was determined to be present as a confined system located approximately 15 to 45 feet below ground surface (bgs). The APH Pond unit is constructed within the sand, silt, and clay that overly the uppermost *usable* aquifer. The bottom of the clay liner is approximately nine feet above the bottom of the confining unit. The clay/silty clay confining layer isolates the APH Pond from the uppermost aquifer and eliminates the potential for a hydraulic connection.

To determine the potential maximum water table, groundwater elevation data from surrounding wells (MW-39, MW-40, MW-41, MW-62, MW-63, and MW-64) was collected between December 2016 and May 2018. Due to the confined conditions, hydrostatic pressure causes the water levels in the surrounding wells to artificially rise above the confining unit. However, the potential rise in groundwater elevations beneath the APH Pond is vertically limited by the presence of the confining layer.

Based on this demonstration, the base of the APH Pond is located greater than five feet above the upper limit of the uppermost aquifer; therefore, the APH Pond is in compliance with the requirements of §257.60.

2.2 §257.61 – Wetlands

The CCR location standards restrict existing and new CCR surface impoundments from being located in wetlands, as defined by 40 CFR 232.2 (40 CFR 257.61(a)). Wetlands are defined in 40 CFR 232.2 definition of *Waters of the United States* (3)(iv) as, "...those areas that are inundated or saturated by surface or groundwater at a frequency and duration sufficient to support, and that under normal circumstances do support, a prevalence of vegetation typically adapted for life in saturated soil conditions. Wetlands generally include swamps, marshes, bogs, and similar areas." TRC reviewed historical aerial photographs and topographic maps to ascertain whether or not the APH Pond is located in a wetland.

Based on a review of historical documentation and Site-specific data, TRC is of the opinion that the APH Pond is not located in an area exhibiting wetland characteristics.

The U.S. Fish and Wildlife Service (USFWS) National Wetlands Inventory (NWI) Mapper (Appendix B) was accessed to evaluate wetland conditions in the vicinity of the APH Pond. The NWI identifies areas exhibiting wetland characteristics and is based on biological attributes visible in aerial imagery.

The USFWS NWI Mapper identified a potential Palustrine Emergent wetland (PEM) on the west end of the APH Pond located within the boundaries of the W.A. Parish Electric Generating Station, however, the NWI mapping imagery is not necessarily ground truthed. Furthermore, the APH Pond does not exhibit the characteristics described by the NWI Mapper. TRC concludes that the APH Pond is not located in wetlands based on the lack of specific wetland characteristics and its industrial use as a CCR surface impoundment, as defined in 40 CFR 232.2.

Evidence of wetlands in the APH Pond area is not supported by this determination; therefore, the APH Pond is not located in a wetland and is in compliance with the requirements of §257.61.

2.3 §257.62 – Fault areas

The federal CCR rule requires that CCR units not be located within 60 meters (200 feet) of the outermost damage zone of a fault that has had displacement in Holocene time (11,700 years ago through the present) unless the owner or operator demonstrates that an alternative setback distance of less than 60 meters (200 feet) will prevent damage to the structural integrity of the CCR unit. To determine recent fault activity in the area, subsurface exploration and regional geologic information was reviewed.

As shown on the USGS Texas Geology Web Map Viewer (Appendix C), no faults have been mapped in the APH Pond area. The closest fault line to the APH Pond is approximately 26 miles to the northeast of the Site.

Evidence of active faulting during the Holocene in the APH Pond area is not supported by this determination; therefore, the APH Pond is not located in an active fault area and is in compliance with the requirements of §257.62.

2.4 §257.63 – Seismic Impact Zones

The federal CCR rule requires that CCR units not be located in seismic impact zones unless the owner or operator demonstrates that all structural components including liners, leachate collection and removal systems, and surface water control systems, are designed to resist the maximum horizontal acceleration in lithified earth material for the Site. The federal CCR rule defines a seismic impact zone as “an area having a 2% or greater probability that the maximum expected horizontal acceleration, expressed as a percentage of the earth’s gravitation pull (g), will exceed 0.10 g in 50 years”.

To determine whether the APH Pond is located in a seismic impact zone, the 2015 National Earthquake Hazards Reduction Program U.S. Seismic Design Maps website (Appendix D) was reviewed. The APH Pond area indicates a mapped peak ground acceleration of 0.084 g. This calculated design peak ground acceleration value is less than 0.10 g in 50 years.

Evidence of a seismic impact zone is not supported by this determination; therefore, the APH Pond is not located in a seismic impact zone and is in compliance with the requirements of §257.63.

2.5 §257.64 – Unstable Areas

The federal CCR rule requires that CCR units not be located in an unstable area unless the owner or operator demonstrates that recognized and generally accepted good engineering practices have been incorporated into the design of the CCR unit to ensure that the integrity of the structural components of the CCR unit will not be disrupted. Factors associated with soil conditions resulting in significant differential settlement, geologic or geomorphologic features, and human-made features or events must be evaluated to determine compliance.

This demonstration was performed by conducting a visual inspection of the Station. Overall, the impoundment was found to be in good condition. The area in and around the facility is very flat topographically. The Plant is located on the southeastern shore of Smithers Lake, with the APH Pond impoundment located within the Plant Area south of Smithers Lake.

The APH Pond is located south of the Plant, adjacent to the coal pile and associated coal pile runoff pond. There is a rail siding located immediately south of the pond. It is an earthen impoundment. The pond is an irregular shape and the perimeter berms are similarly irregular, varying in width and height.

The berms are well vegetated except on the interior side of the south berm and exterior side of the north berm, both of which appear to have been recently re-graded due to the presence of equipment tracks in the surface soil. No stressed vegetation or erosion was noted during the inspection. The irregularities in berm dimensions appear to be a function of construction rather than post-construction settlement, slippage, or bulging. No seeps or animal burrows were observed.

Evidence of unstable areas was not observed during the inspection of the APH Pond impoundment; therefore, the APH Pond is in compliance with the requirements of §257.64.

Section 3

Conclusions

Based on the evaluation provided in this demonstration, the APH Pond at the W.A. Parish Electric Generating Station is in compliance with the location restrictions provided in §257.60 through §257.64 of the CCR rule. No additional action, justification, or demonstration is required to document compliance with the location restrictions provided in the CCR rule after this demonstration has been placed into the operating record, posted to the publicly accessible website, and provided for government notification.

Section 4

References

BEG 1982. Geologic Atlas of Texas, Houston Sheet. The University of Texas at Austin, Bureau of Economic Geology. Revised 1982.

Fort Bend County, Texas. Floodplain Mapping Tool. Accessed [9/17/2018].

Google Earth Pro. Available online at <https://earth.google.com/download-earth.html>. Accessed [9/14/2018].

Sargent & Lundy LLC, Liner Documentation for Existing CCR Surface Impoundments. October 7, 2016.

Thorkildsen, David, Texas Water Development Board. Evaluation of Water Resources of Fort Bend County, Texas, Report 321. January 1990.

United States Geological Survey (USGS). 2015. U.S. Seismic Design Maps: 2015 National Earthquake Hazards Reduction Program Provisions. Available online at <http://earthquake.usgs.gov/designmaps/beta/us/>. Accessed [9/11/2018].

U.S. Fish and Wildlife Service National Wetlands Inventory (NWI). 2018. "Wetland Mapper." Available online at <https://www.fws.gov/wetlands/data/mapper.html>. Accessed [9/14/2018].

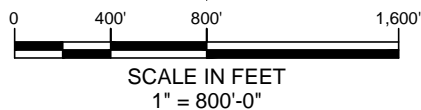
United States Geological Survey (USGS). 2018. USGS - Texas Geology Web Map Viewer: Available online at <https://txpub.usgs.gov/dss/texasgeology/>. Accessed [9/14/2018].




IMAGERY SOURCE: Google Earth (10/28/2017)



TEXAS
SUBJECT SITE LOCATION



PROJECT:		NRG TEXAS POWER, LLC W.A. Parish Station Thompsons, Texas	
TITLE:		AIR PREHEATER POND	
DRAWN BY:	O. Fonseka	PROJECT No.:	294645.0000.0000
CHECKED BY:	T. Dworaczyk	FIGURE 2	
APPROVED BY:	T. Dworaczyk		
DATE:	September 2018	 10550 Richmond Ave. Suite 210 Houston, TX 77042 Phone: 713.244.1000	
FILE:	Fig 2 - NRG-WA Parish Station - Air Preheater Pond.dwg		

Appendix A

Photographs



Location: SE Corner Side APH Pond facing SW

Site: WA Parish Owner: NRG

Photograph Taken by: Jason Leik Date of Inspection: 9/11/18



Location: SE Corner Side APH Pond facing W

Site: WA Parish Owner: NRG

Photograph Taken by: Jason Leik Date of Inspection: 9/11/18



Location: SE Corner Side APH Pond facing SW

Site: WA Parish Owner: NRG

Photograph Taken by: Jason Leik Date of Inspection: 9/11/18



Location: SE Corner Side APH Pond facing NW

Site: WA Parish Owner: NRG

Photograph Taken by: Jason Leik Date of Inspection: 9/11/18



Location: SE Corner Side APH Pond facing NW

Site: WA Parish Owner: NRG

Photograph Taken by: Jason Leik Date of Inspection: 9/11/18



Location: NW Corner Side APH Pond facing S

Site: WA Parish Owner: NRG

Photograph Taken by: Jason Leik Date of Inspection: 9/11/18



Location: NW Corner Side APH Pond facing NW

Site: WA Parish Owner: NRG

Photograph Taken by: Jason Leik Date of Inspection: 9/11/18



Location: W Side of APH Pond Facing S

Site: WA Parish Owner: NRG

Photograph Taken by: Jason Leik Date of Inspection: 9/11/18

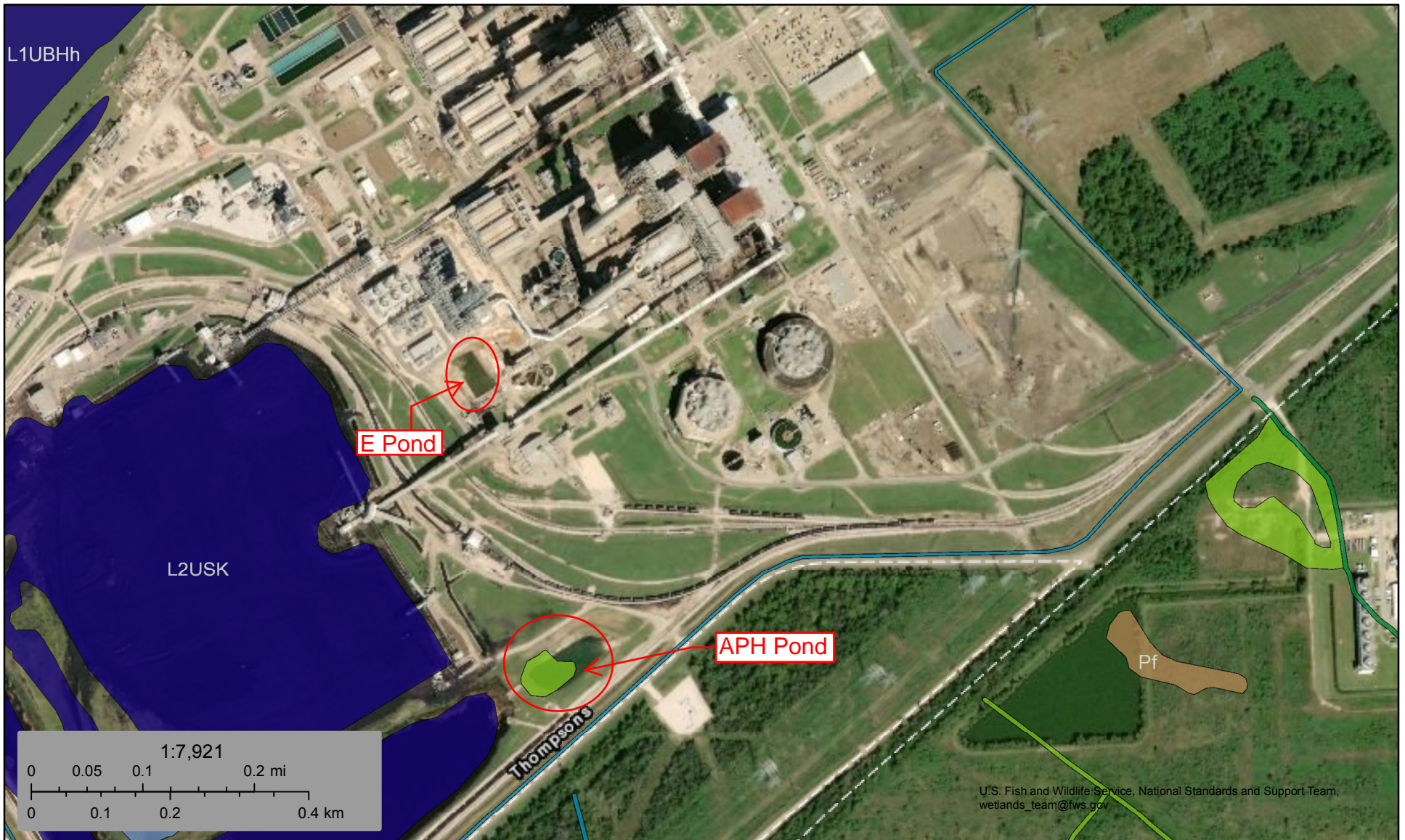


Location: W Side APH Pond Discharge Channel to Coal Pile Runoff Pond

Site: WA Parish Owner: NRG

Photograph Taken by: Jason Leik Date of Inspection: 9/11/18

Appendix B
National Wetlands Inventory Mapper



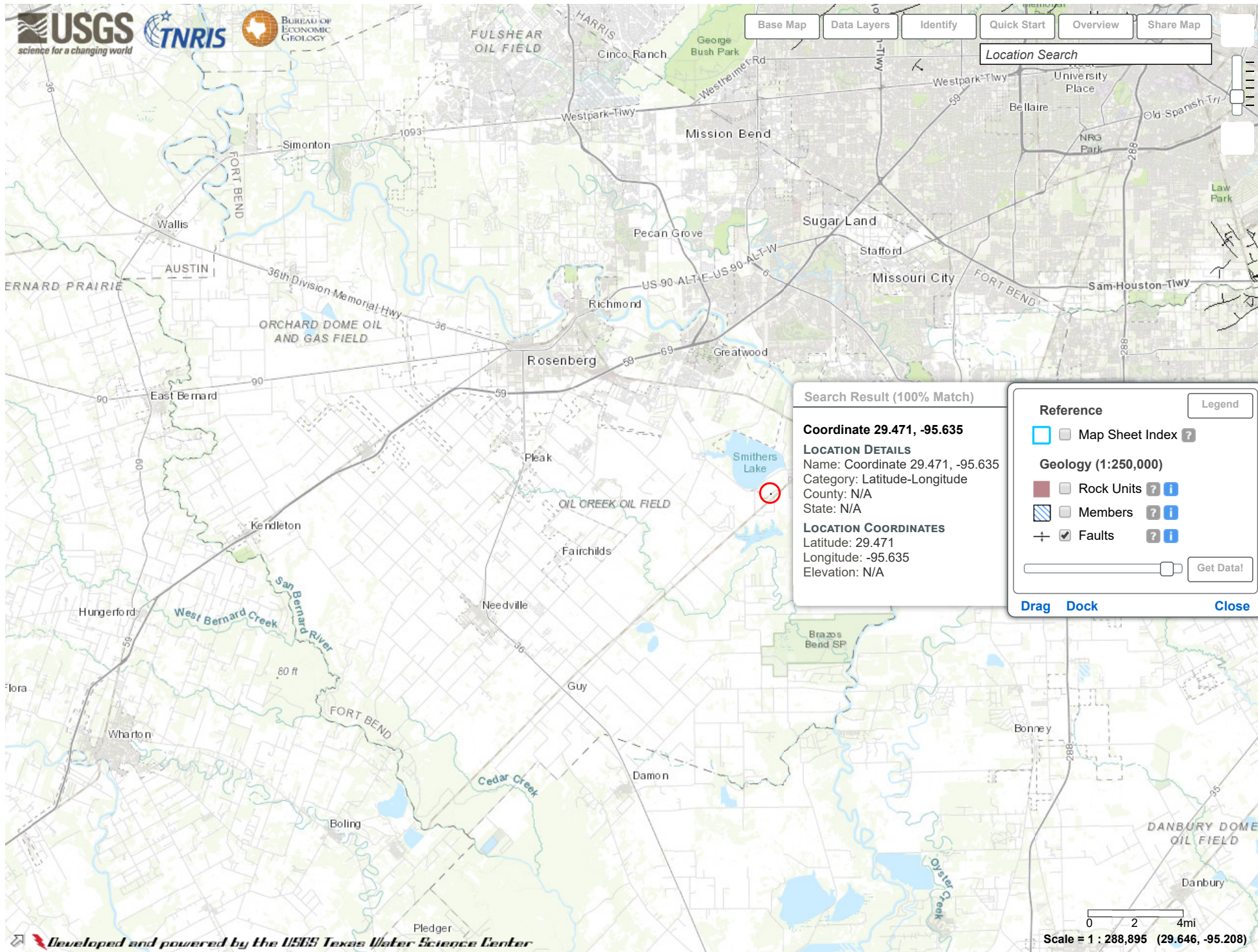
September 14, 2018

Wetlands

- | | | | | | |
|---|--------------------------------|---|-----------------------------------|---|-------|
|  | Estuarine and Marine Deepwater |  | Freshwater Emergent Wetland |  | Lake |
|  | Estuarine and Marine Wetland |  | Freshwater Forested/Shrub Wetland |  | Other |
|  | Freshwater Pond |  | Riverine | | |

This map is for general reference only. The US Fish and Wildlife Service is not responsible for the accuracy or currentness of the base data shown on this map. All wetlands related data should be used in accordance with the layer metadata found on the Wetlands Mapper web site.

Appendix C
USGS Texas Geology Web Map Viewer



Appendix D
U.S. Seismic Design Map

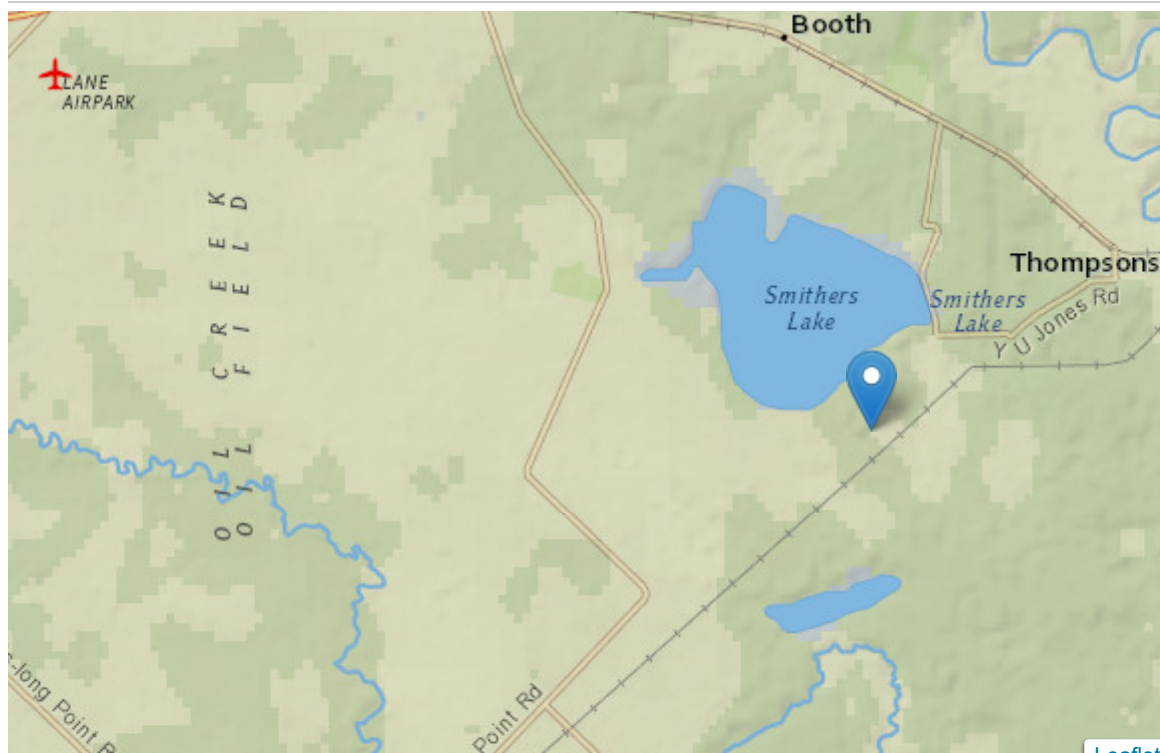
U.S. Geological Survey - Earthquake Hazards Program

i Due to insufficient resources and the recent development of similar web tools by third parties, this spring the USGS will be streamlining the two U.S. Seismic Design Maps web applications, including the one below. Whereas the current applications each interact with users through a graphical user interface (GUI), the new web services will receive the inputs (e.g. latitude and longitude) in the form of a web address and return the outputs (e.g. S_{DS} and S_{D1}) in text form, without supplementary graphics. Though designed primarily to be read by the aforementioned third-party web GUIs, the text outputs are also human-readable. To preview the new web services, [please click here](#). Step-by-step instructions for using one of these web services, namely that for the recently published 2016 ASCE 7 Standard, [are posted here](#).

APH Pond-Parish

Latitude = 29.471°N, Longitude = 95.635°W

Location



<https://earthquake.usgs.gov/designmaps/beta/us/>

Reference Document

2015 NEHRP Provisions

Site Class

C: Very Dense Soil and Soft Rock

Risk Category

I or II or III



$S_S = 0.064 \text{ g}$

$S_{MS} = 0.084 \text{ g}$

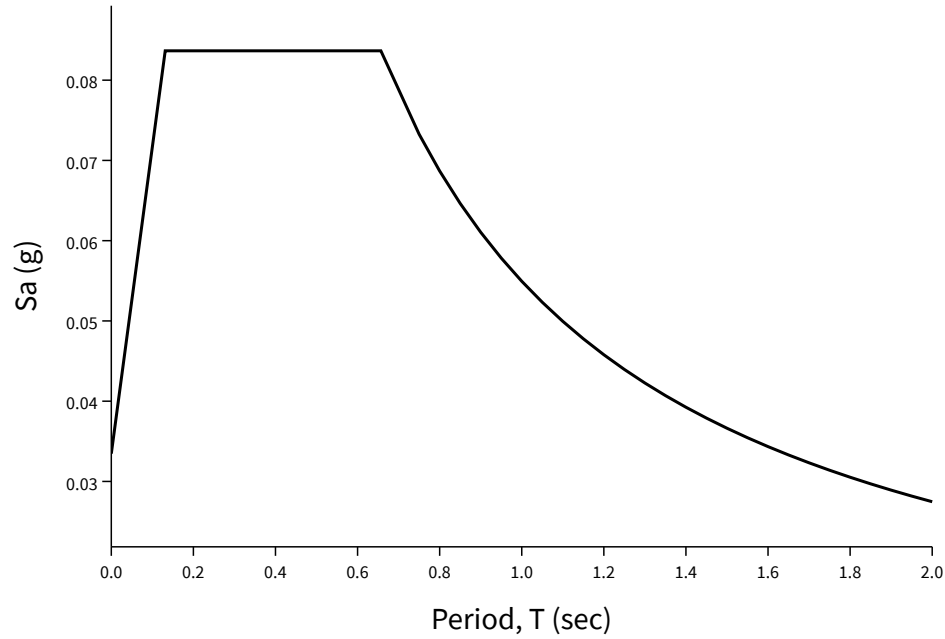
$S_{DS} = 0.056 \text{ g}$

$S_1 = 0.037 \text{ g}$

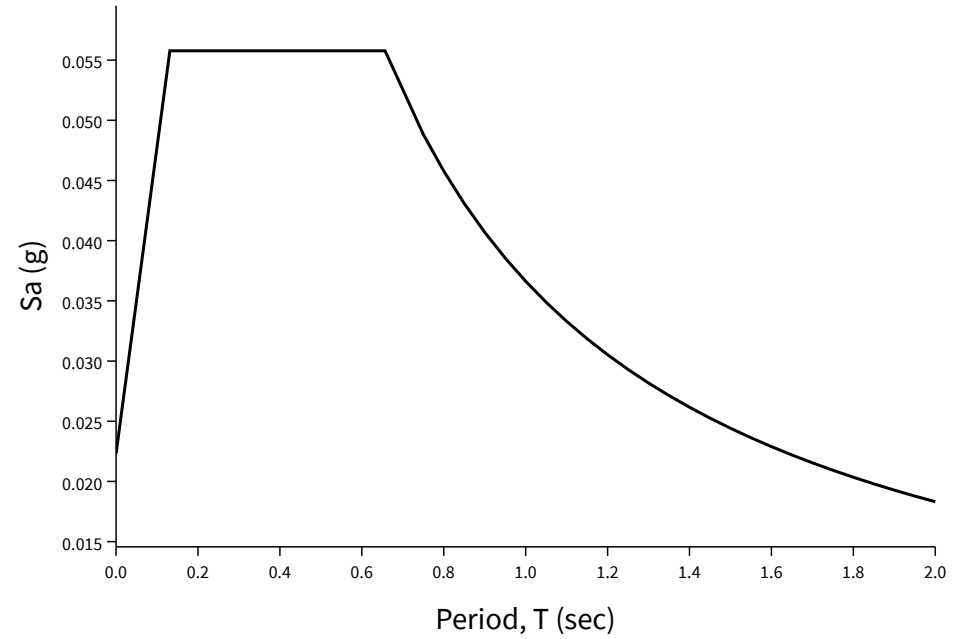
$S_{M1} = 0.055 \text{ g}$

$S_{D1} = 0.037 \text{ g}$

MCE_R Spectrum



Design Response Spectrum



Mapped Acceleration Parameters, Long-Period Transition Periods, and Risk Coefficients

Note: The S_5 and S_1 ground motion maps provided below are for the direction of maximum horizontal spectral response acceleration. They have been converted from corresponding geometric mean ground motions computed by the USGS by applying factors of 1.1 (to obtain S_5) 1.3 (to obtain S_1).

- [FIGURE 22-1 \$S_5\$ Risk-Targeted Maximum Considered Earthquake \(\$MCE_R\$ \) Ground Motion Parameter for the Conterminous United States for 0.2 s Spectral Response Acceleration \(5% of Critical Damping\), Site Class B](#)
- [FIGURE 22-2 \$S_1\$ Risk-Targeted Maximum Considered Earthquake \(\$MCE_R\$ \) Ground Motion Parameter for the Conterminous United States for 1.0 s Spectral Response Acceleration \(5% of Critical Damping\), Site Class B](#)
- [FIGURE 22-9 Maximum Considered Earthquake Geometric Mean \(\$MCE_G\$ \) PGA, %g, Site Class B for the Conterminous United States](#)
- [FIGURE 22-14 Mapped Long-Period Transition Period, \$T_L\$ \(s\), for the Conterminous United States](#)
- [FIGURE 22-18 Mapped Risk Coefficient at 0.2 s Spectral Response Period, \$C_{RS}\$](#)
- [FIGURE 22-19 Mapped Risk Coefficient at 1.0 s Spectral Response Period, \$C_{R1}\$](#)

Site Class

The authority having jurisdiction (not the USGS), site-specific geotechnical data, and/or the default has classified the site class as Site Class , based on the site soil properties in accordance with Chapter 20.

Table 20.3-1 Site Classification

Site Class	\bar{v}_s	\bar{N} or \bar{N}_{ch}	\bar{s}_u
A. Hard Rock	>5,000 ft/s	N/A	N/A
B. Rock	2,500 to 5,000 ft/s	N/A	N/A
C. Very dense soil and soft rock	1,200 to 2,500 ft/s	>50	>2,000 psf
D. Stiff Soil	600 to 1,200 ft/s	15 to 50	1,000 to 2,000 psf
E. Soft clay soil	<600 ft/s	<15	<1,000 psf
	Any profile with more than 10 ft of soil having the characteristics: <ul style="list-style-type: none"> • Plasticity index $PI > 20$ • Moisture content $w \geq 40\%$, and • Undrained shear strength $\bar{s}_u < 500$ psf 		
F. Soils requiring site response analysis in accordance with Section 21.1	See Section 20.3.1		
For SI: 1ft/s = 0.3048 m/s 1lb/ft ² = 0.0479 kN/m ²			

Site Coefficients and Risk-Targeted Maximum Considered Earthquake (MCE_R) Spectral Response Acceleration Parameters

Risk-targeted Ground Motion (0.2 s)

$$C_{RS}S_{SUH} = 0.952 \times 0.068 = 0.064 \text{ g}$$

Deterministic Ground Motion (0.2 s)

$$S_{SD} = 1.500 \text{ g}$$

$$S_S \equiv \text{“Lesser of } C_{RS}S_{SUH} \text{ and } S_{SD}\text{”} = 0.064 \text{ g}$$

Risk-targeted Ground Motion (1.0 s)

$$C_{R1}S_{1UH} = 0.888 \times 0.041 = 0.037 \text{ g}$$

Deterministic Ground Motion (1.0 s)

$$S_{1D} = 0.600 \text{ g}$$

$$S_1 \equiv \text{“Lesser of } C_{R1}S_{1UH} \text{ and } S_{1D}\text{”} = 0.037 \text{ g}$$

Table 11.4-1: Site Coefficient F_a

Site Class	Spectral Reponse Acceleration Parameter at Short Period					
	$S_S \leq 0.25$	$S_S = 0.50$	$S_S = 0.75$	$S_S = 1.00$	$S_S = 1.25$	$S_S \geq 1.50$
A	0.8	0.8	0.8	0.8	0.8	0.8
B (measured)	0.9	0.9	0.9	0.9	0.9	0.9
B (unmeasured)	1.0	1.0	1.0	1.0	1.0	1.0

Site Class	Spectral Reponse Acceleration Parameter at Short Period					
	$S_S \leq 0.25$	$S_S = 0.50$	$S_S = 0.75$	$S_S = 1.00$	$S_S = 1.25$	$S_S \geq 1.50$
C	1.3	1.3	1.2	1.2	1.2	1.2
D (determined)	1.6	1.4	1.2	1.1	1.0	1.0
D (default)	1.6	1.4	1.2	1.2	1.2	1.2
E	2.4	1.7	1.3	1.2 [*]	1.2 [*]	1.2 [*]
F	See Section 11.4.7					

* For Site Class E and $S_S \geq 1.0$ g, see the requirements for site-specific ground motions in Section 11.4.7 of the 2015 NEHRP Provisions. Here the exception to those requirements allowing F_a to be taken as equal to that of Site Class C has been invoked.

Note: Use straight-line interpolation for intermediate values of S_S .

Note: Where Site Class B is selected, but site-specific velocity measurements are not made, the value of F_a shall be taken as 1.0 per Section 11.4.2.

Note: Where Site Class D is selected as the default site class per Section 11.4.2, the value of F_a shall not be less than 1.2 per Section 11.4.3.

For Site Class = C and $S_S = 0.064$ g, $F_a = 1.300$

Table 11.4-2: Site Coefficient F_v

Site Class	Spectral Response Acceleration Parameter at 1-Second Period					
	$S_1 \leq 0.10$	$S_1 = 0.20$	$S_1 = 0.30$	$S_1 = 0.40$	$S_1 = 0.50$	$S_1 \geq 0.60$
A	0.8	0.8	0.8	0.8	0.8	0.8
B (measured)	0.8	0.8	0.8	0.8	0.8	0.8
B (unmeasured)	1.0	1.0	1.0	1.0	1.0	1.0
C	1.5	1.5	1.5	1.5	1.5	1.4
D (determined)	2.4	2.2 ¹	2.0 ¹	1.9 ¹	1.8 ¹	1.7 ¹
D (default)	2.4	2.2 ¹	2.0 ¹	1.9 ¹	1.8 ¹	1.7 ¹
E	4.2	3.3 ¹	2.8 ¹	2.4 ¹	2.2 ¹	2.0 ¹
F	See Section 11.4.7					

¹ For Site Class D or E and $S_1 \geq 0.2$ g, site-specific ground motions might be required. See Section 11.4.7 of the 2015 NEHRP Provisions.

Note: Use straight-line interpolation for intermediate values of S_1 .

Note: Where Site Class B is selected, but site-specific velocity measurements are not made, the value of F_v shall be taken as 1.0 per Section 11.4.2.

For Site Class = C and $S_1 = 0.037$ g, $F_v = 1.500$

Site-adjusted MCE_R (0.2 s)

$$S_{MS} = F_a S_S = 1.300 \times 0.064 = 0.084 \text{ g}$$

Site-adjusted MCE_R (1.0 s)

$$S_{M1} = F_v S_1 = 1.500 \times 0.037 = 0.055 \text{ g}$$

Design Spectral Acceleration Parameters

Design Ground Motion (0.2 s)

$$S_{DS} = \frac{2}{3} S_{MS} = \frac{2}{3} \times 0.084 = 0.056 \text{ g}$$

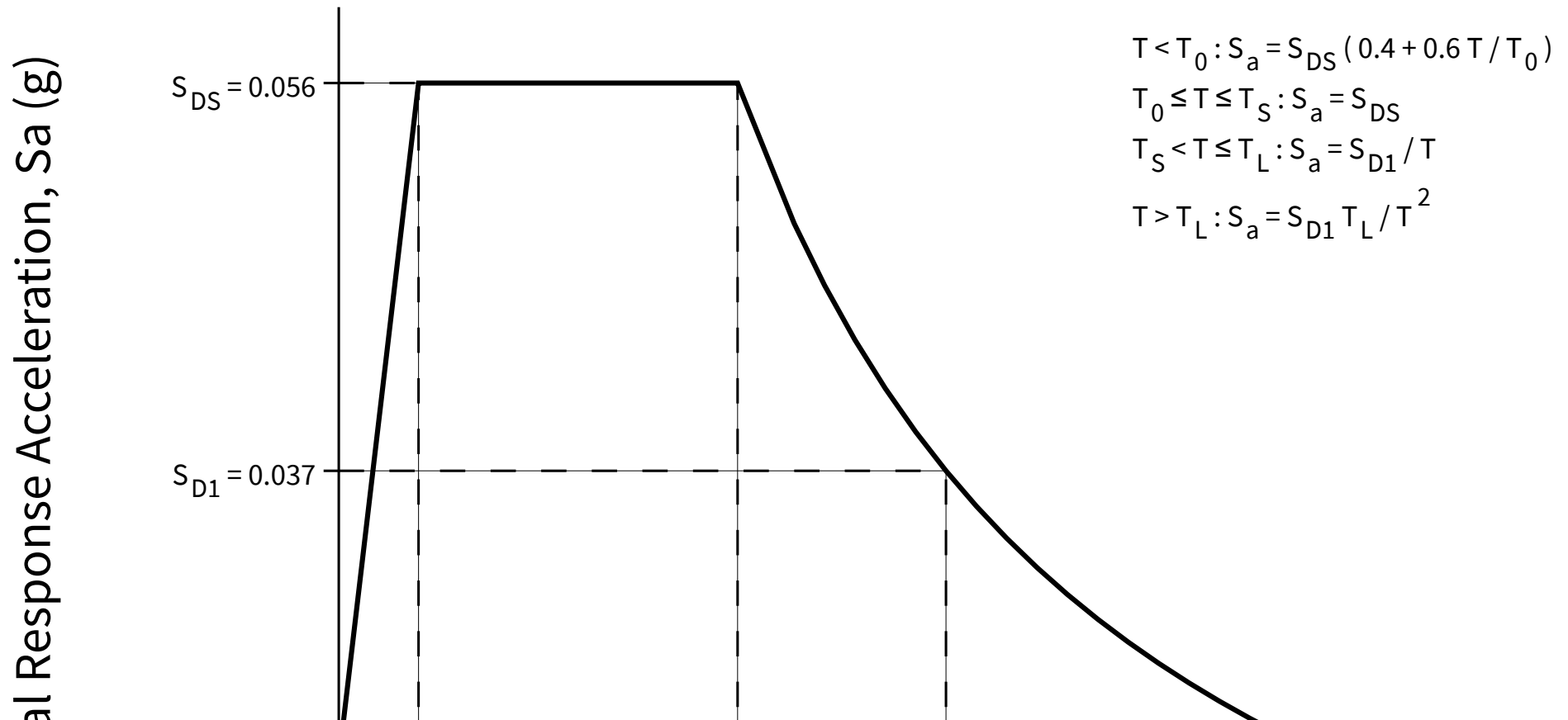
Design Ground Motion (1.0 s)

$$S_{D1} = \frac{2}{3} S_{M1} = \frac{2}{3} \times 0.055 = 0.037 \text{ g}$$

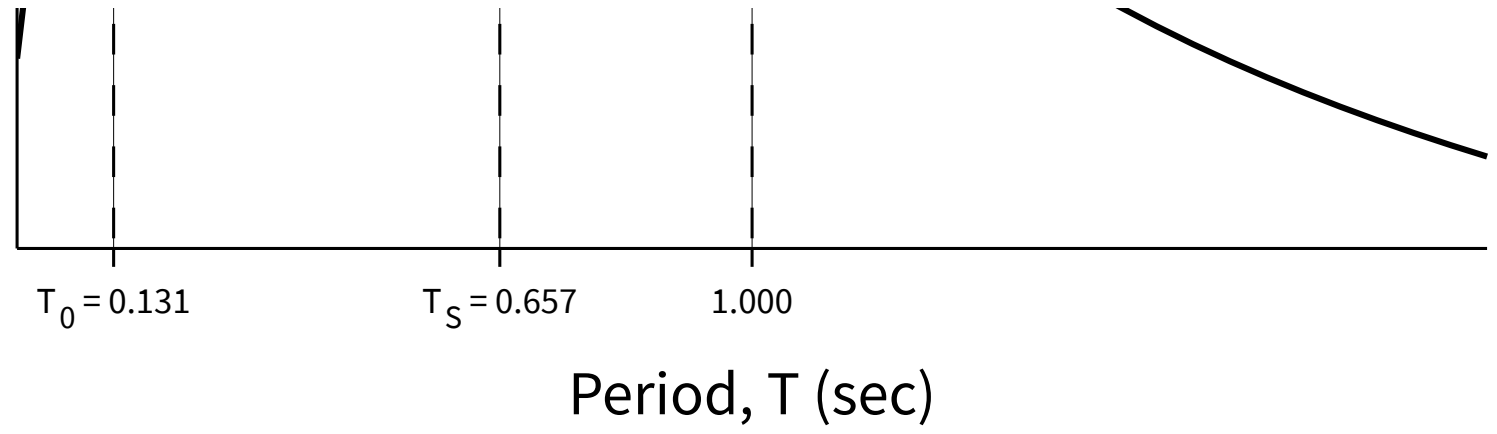
Design Response Spectrum

Long-Period Transition Period = $T_L = 12$ s

Figure 11.4-1: Design Response Spectrum

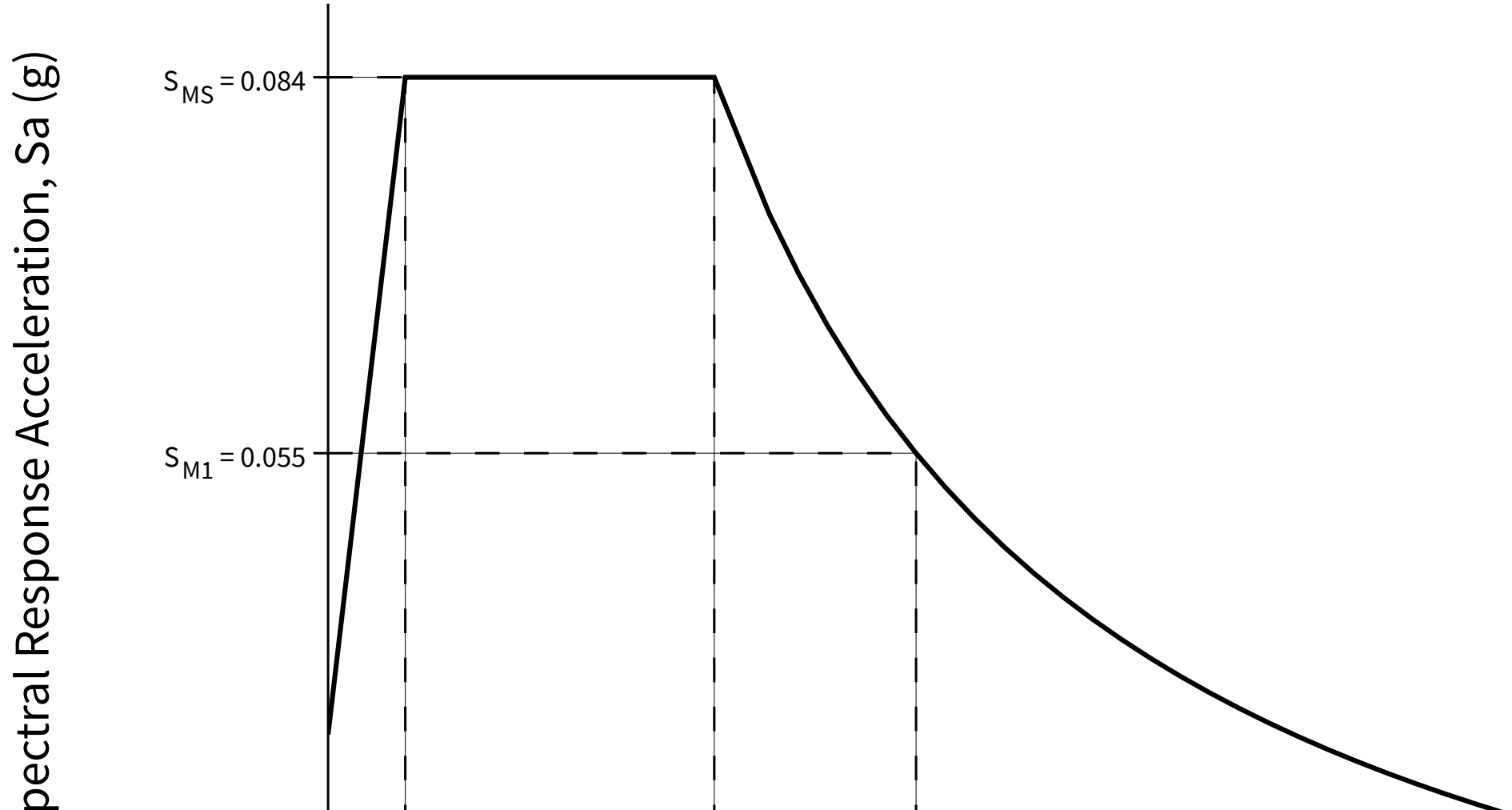


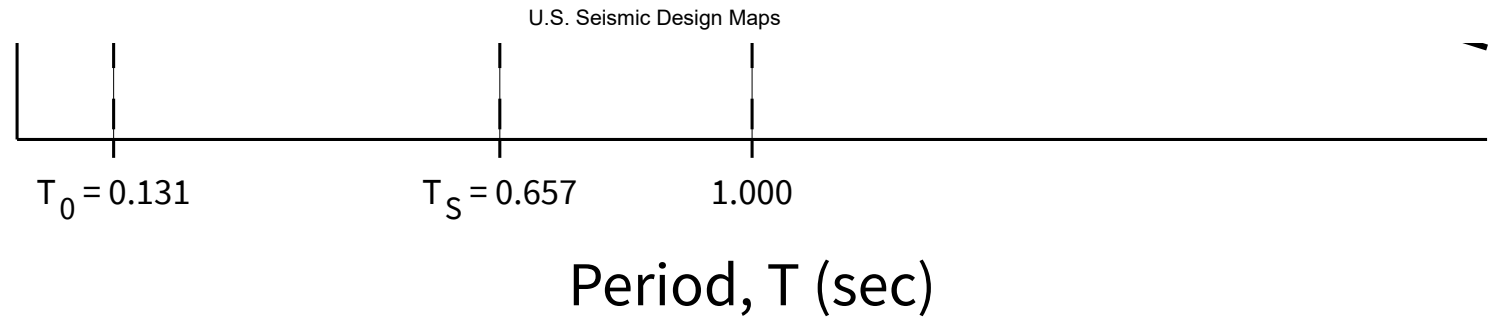
Spectra



MCE_R Response Spectrum

The MCE_R response spectrum is determined by multiplying the design response spectrum above by 1.5.





Additional Geotechnical Investigation Report Requirements for Seismic Design Categories D through F

Table 11.8-1: Site Coefficient for F_{PGA}

Site Class	Mapped MCE Geometric Mean (MCE_G) Peak Ground Acceleration					
	PGA ≤ 0.10	PGA = 0.20	PGA = 0.30	PGA = 0.40	PGA = 0.50	PGA ≥ 0.60
A	0.8	0.8	0.8	0.8	0.8	0.8
B (measured)	0.9	0.9	0.9	0.9	0.9	0.9
B (unmeasured)	1.0	1.0	1.0	1.0	1.0	1.0
C	1.3	1.2	1.2	1.2	1.2	1.2
D (determined)	1.6	1.4	1.3	1.2	1.1	1.1
D (default)	1.6	1.4	1.3	1.2	1.2	1.2
E	2.4	1.9	1.6	1.4	1.2	1.1
F	See Section 11.4.7					

Note: Use straight-line interpolation for intermediate values of PGA

Note: Where Site Class D is selected as the default site class per Section 11.4.2, the value of F_{pga} shall not be less than 1.2.

For Site Class = C and PGA = 0.031 g, $F_{PGA} = 1.300$

Mapped MCE_G

PGA = 0.031 g

Site-adjusted MCE_G

$PGA_M = F_{PGA}PGA = 1.300 \times 0.031 = 0.040$ g

