

WEEKLY SUMMARY REPORT

Week Ending: 08/29/2024 (Thursday)

Report No.: W-03-082924

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Civil & Environmental Consultants, Inc.

PROJECT INFORMATION

PROJECT NAME:	BYPASS BASIN RETROFIT CQA		
LOCATION:	Powerton Generating Station	CEC PROJECT NO:	343-014.0200
PLANS AND SPECS:	S&L Bypass Basin Retrofit Project	WEATHER:	0.7" of rain
ISSUED DATE:	07/12/24	TEMP. RANGE (°F)	71-97

PERSONNEL

FIELD REP(S):	Derek Dorsz	CEC PROJ. MANAGER(S):	Dean Jones
CLIENT:	Midwest Generation	CLIENT CONTACT(S):	Joe Kotas
CONTRACTOR:	Bluff City Construction (BCC)	SUPERVISOR:	Larry Hunt
CONTRACTOR:	Clean Air and Water Systems (CAAWS)	SUPERVISOR:	Andy Khamarlorm

SAFETY MEETINGS AND PARTICIPATION

Participation in Contractor's Tailgate Safety Meeting? Yes No Vehicle Check Performed? Yes or No
Plan for the Day Required (WSM 200.25): Yes (CLICK LINK BELOW) No

WORK PERFORMED SINCE CEC'S LAST VISIT⁽¹⁾

Work performed since CEC representative's last site visit? ⁽¹⁾ Yes No
Date CEC representative was last onsite: **Saturday, August 24, 2024**
(1) Critical work or work requiring continuous observation that has been completed without CEC representation being present onsite. CEC was not made aware that this work was being completed.

SUMMARY OF WORK OBSERVED, LOCATION, AND CONTRACTOR PERFORMING WORK

- CAAWS completed geomembrane detail work and encompassed the site in the liner, including attaching liner to concrete structures.
- CAAWS silicon sealed the liner edge along inlet structure and existing marker posts.
- CAAWS began and completed deployment of drainage geocomposite liner across the basin.
- BCC completed excavation of anchor trench along the perimeter.
- BCC completed backfill of anchor trench.
- BCC installed upper sane cushion layer along the west and northwest corner of basin.
- BCC began installation of 12" sand filer layer at the ramp and southern half of basin.

UNEXPECTED, UNUSUAL, OR NONCONFORMING OBSERVATIONS (NEW / RESOLVED)

Unexpected, unusual, or nonconforming work observed? Yes No

- N/A

SUMMARY OF MEETINGS / DISCUSSIONS / PHONE CONVERSATIONS

- Geotextile is to be installed beneath the road surfacing aggregate within areas to have regular traffic.

ATTACHMENTS

- Field Density Test Report and Sand Cone Test Reports.
- Flowable Fill test results.

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DESCRIPTION OF SAMPLES TAKEN OR MATERIALS DELIVERED TO LAB

- PBBR-Sand-1 collected and taken to lab.
- PBBR-AT-1 collected and taken to lab.

PHOTOGRAPHS



Photo 1: shows deployment of drainage geocomposite and completion of geomembrane installation.



Photo 2: shows completion of drainage geocomposite installation.

* No representations or warranties are made regarding the accuracy of the information generated by the Theodolite application, which is stamped on the photo, or the suitability of that information for any; legal, engineering, surveying, or other use or purpose.

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Date & Time: Tue, Aug 27, 2024 at 15:18:33 CDT
Position: +040.542974° / -089.675951° (±15.1ft)
Altitude: 490ft (±11.7ft)
Datum: WGS-84
Azimuth/Bearing: 156° S24E 2773mils True (±13°)
Elevation Grade: -008%
Horizon Grade: -004%
Zoom: 1.0X
CEC

Photo 3: shows compaction of anchor trench backfill.



Date & Time: Thu, Aug 29, 2024 at 11:27:33 CDT
Position: +040.542391° / -089.676277° (±15.1ft)
Altitude: 464ft (±11.7ft)
Datum: WGS-84
Azimuth/Bearing: 002° N02E 0036mils True (±14°)
Elevation Grade: -015%
Horizon Grade: +001%
Zoom: 1.0X
CEC

Photo 4: shows placement of sand filter layer.

APPROVED BY

FIELD REP: Derek Dorsz DATE: 08/29/2024 CEC MANAGER: Dean Jones DATE: 09/04/2024

This document is draft until reviewed and approved by a Project Manager

NOTICE: Our firm's professionals are represented onsite solely to observe operations of the contractor identified to form opinions about the adequacy of those operations and to report those opinions to our client. The presence and activities of our field representative do not relieve the contractor from its obligation to meet contractual requirements. The contractor retains sole responsibility for site safety and the methods operations and sequences of construction.

* No representations or warranties are made regarding the accuracy of the information generated by the Theodolite application, which is stamped on the photo, or the suitability of that information for any; legal, engineering, surveying, or other use or purpose.

DAILY FIELD REPORT

Date: 08/27/2024 (Tuesday)

Report No.: 14-082724

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Civil & Environmental Consultants, Inc.

NUCLEAR COMPACTION TEST DATA (ASTM D6938)

PROJECT NAME: Powerton Bypass Basin Retrofit PROJECT NUMBER: 343-014
CEC TECHNICIAN: Derek Dorsz GAUGE NUMBER: 27636
CONTRACTOR: Bluff City Construction

STANDARD COUNTS:

DENSITY 954 % DEVIATION 0.73 ±1% PASSING
MOISTURE 613 % DEVIATION 0.33 ±2% PASSING

TEST NUMBER	17	18	19	20	21	22
LOCATION	N: 1411829.1 E: 2432881.7	N: 1411992.6 E: 2432965.1	N: 412100.8 E: 2432858.7	N: 1411829.1 E: 2432881.7	N: 1411992.6 E: 2432965.1	N: 1412100.8 E: 2432858.7
ELEVATION OR LIFT NUMBER	Lift 1 Z: 465.3	Lift 1 Z: 465.6	Lift 1 Z: 466.1	Lift 2 Z: 465.8	Lift 2 Z: 466.1	Lift 2 Z: 466.6
LIFT THICKNESS (in.)	6	6	6	6	6	6
NUMBER OF PASSES	4	4	4	4	4	4
PROBE DEPTH (in.)	0	0	0	0	0	0
FIELD WET DENSITY (pcf)	131.0	131.6	131.5	130.2	131.5	132.0
FIELD DRY DENSITY (pcf)	113.0	113.5	114.2	114.3	114.6	114.7
COMPACTION (%)	95.1	95.5	96.1	96.2	96.5	96.5
COMPACTION PASS/FAIL	PASS	PASS	PASS	PASS	PASS	PASS
FIELD MOISTURE (%)	13.7	13.8	13.2	12.2	12.9	13.1
MOISTURE PASS/FAIL	PASS	PASS	PASS	PASS	PASS	PASS
LAB PROCTOR MDD	118.8	118.8	118.8	118.8	118.8	118.8
PROCTOR TYPE (Mod./Stan.)	Modified	Modified	Modified	Modified	Modified	Modified
SPECIFIED MIN. COMPACTION (%)	95.0	95.0	95.0	95.0	95.0	95.0
LABORATORY OMC (%)	13.0	13.0	13.0	13.0	13.0	13.0
SPECIFIED MOISTURE RANGE	10.0 – 16.0	10.0 – 16.0	10.0 – 16.0	10.0 – 16.0	10.0 – 16.0	10.0 – 16.0
RETEST NUMBER	N/A	N/A	N/A	N/A	N/A	N/A
REMARKS						

DAILY FIELD REPORT

Date: 08/27/2024 (Tuesday)

Report No.: 14-082724

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Civil & Environmental Consultants, Inc.

TEST NUMBER	23	24	25			
LOCATION	N: 1411829.1 E: 2432881.7	N: 1411992.6 E: 2432965.1	N: 1412100.8 E: 2432858.7			
ELEVATION OR LIFT NUMBER	Lift 3 Z: 466.3	Lift 3 Z: 466.6	Lift 3 Z: 467.1			
LIFT THICKNESS (in.)	6	6	6			
NUMBER OF PASSES	4	4	4			
PROBE DEPTH (in.)	0	0	0			
FIELD WET DENSITY (pcf)	133.2	132.9	133.2			
FIELD DRY DENSITY (pcf)	115.2	114.8	115.2			
COMPACTION (%)	97.0	96.6	97.0			
COMPACTION PASS/FAIL	PASS	PASS	PASS			
FIELD MOISTURE (%)	13.5	13.6	13.5			
MOISTURE PASS/FAIL	PASS	PASS	PASS			
LAB PROCTOR MDD	118.8	118.8	118.8			
PROCTOR TYPE (Mod./Stan.)	Modified	Modified	Modified			
SPECIFIED MIN. COMPACTION (%)	95.0	95.0	95.0			
LABORATORY OMC (%)	13.0	13.0	13.0			
SPECIFIED MOISTURE RANGE	10.0 – 16.0	10.0 – 16.0	10.0 – 16.0			
RETEST NUMBER	N/A	N/A	N/A			
REMARKS						

NOTES:

1. MDD denotes Maximum Dry Density.
2. OMC denotes Optimum Moisture Content.
3. Elevations and lift thicknesses are approximate.
4. N denote Northing, E denotes Easting, and Z denotes elevation. Coordinates given by Bluff City Construction.

Powerton Bypass Basin Retrofit
Sand Cone Test



Sand Cone Analysis (Cannot be performed in soils with coarse aggregates)

Date: 8/27/24 Test No: 25

This method is using balance scales measuring mass to determine unit weight. If you are using digital scales or scales with springs for the force-measuring mechanism, use the "Pounds-Force" Tab.

Obtain Sample for Wet Density

- Excavate hole with large metal spoon or garden trowel that is 3 to 4" in diameter and 3 to 4 inches deep in order to obtain a sample of at least 500 grams (1.1 lbs).

- Fill the cone on the apparatus with the sand from the sandcone to determine the weight of the sand in the cone.

-Weight of sand from full cone (W₁) in Pounds (grams) = 1573.000 grams
-Weight of cone/jar apparatus with sand = (W) in grams 6597.000 grams
-Enter pre-determined density of sand in jar = 1.530 g/cm³

-Place base plate over excavated hole with hole in base plate in center of excavated hole.

-Weigh moist/wet soil from excavation W_s = 1702.000 grams

- Place cone (attached to sand jar) over hole and open valve.

- Turn Valve off and remove cone and base plate. If excavation is not full, spread sand evenly in excavation and repeat last step.

- Determine Volume of Hole Excavated.

- Weigh jar and remaining sand (W₂) 3820.000 grams + weight of sand in the full cone (W₁) and subtract from weight of full jar (W): W - (W₁+W₂) 1204.000 grams

Volume of Excavation = 786.928 cm³

Wet Density of Excavated Soil W_w = 2162.841 kg/m³

Dry Density of Soil

- Place moist soil obtained from the excavation into pan for drying after weighing on the scale

- Place material in iron skillet over active burner and break apart the core so that it can be dried over the heat.

- Carefully remove dried material from skillet making sure to transfer all material into a tin or other apparatus to be weighed.

- Obtain weight in grams of dry soil W_d = 1390.000 grams

- Moisture Content of sample = 18.331 %

Soil Dry Density = 1827.783 kg/m³

***Soil Dry Unit Wt. = 114.05 lb/ft³ *** measured in force (lb/ft³)

DAILY FIELD REPORT

Date: 08/28/2024 (Wednesday)

Report No.: 15-082824

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Civil & Environmental Consultants, Inc.

NUCLEAR COMPACTION TEST DATA (ASTM D6938)

PROJECT NAME: Powerton Bypass Basin Retrofit PROJECT NUMBER: 343-014
CEC TECHNICIAN: Derek Dorsz GAUGE NUMBER: 27636
CONTRACTOR: Bluff City Construction

STANDARD COUNTS:

DENSITY $\frac{954}{613}$ % DEVIATION $\frac{0.73}{0.33}$ ±1% PASSING
MOISTURE $\frac{954}{613}$ % DEVIATION $\frac{0.73}{0.33}$ ±2% PASSING

TEST NUMBER	26	27	28			
LOCATION	N: 1411942.7 E: 2432791.2	N: 1411942.7 E: 2432791.2	N: 1411942.7 E: 2432791.2			
ELEVATION OR LIFT NUMBER	Lift 1 Z: 466.2	Lift 2 Z: 466.7	Lift 3 Z: 467.2			
LIFT THICKNESS (in.)	6	6	6			
NUMBER OF PASSES	4	4	4			
PROBE DEPTH (in.)	0	0	0			
FIELD WET DENSITY (pcf)	135.6	135.7	135.9			
FIELD DRY DENSITY (pcf)	115.6	114.9	114.5			
COMPACTION (%)	97.3	96.7	96.4			
COMPACTION PASS/FAIL	PASS	PASS	PASS			
FIELD MOISTURE (%)	14.7	15.3	15.7			
MOISTURE PASS/FAIL	PASS	PASS	PASS			
LAB PROCTOR MDD	118.8	118.8	118.8			
PROCTOR TYPE (Mod./Stan.)	Modified	Modified	Modified			
SPECIFIED MIN. COMPACTION (%)	95.0	95.0	95.0			
LABORATORY OMC (%)	13.0	13.0	13.0			
SPECIFIED MOISTURE RANGE	10.0 – 16.0	10.0 – 16.0	10.0 – 16.0			
RETEST NUMBER	N/A	N/A	N/A			
REMARKS						

NOTES:

1. MDD denotes Maximum Dry Density.
2. OMC denotes Optimum Moisture Content.
3. Elevations and lift thicknesses are approximate.
4. N denote Northing, E denotes Easting, and Z denotes elevation. Coordinates given by Bluff City Construction.

Powerton Bypass Basin Retrofit
Sand Cone Test



Sand Cone Analysis (Cannot be performed in soils with coarse aggregates)

Date: 8/28/24 Test No: 28

This method is using balance scales measuring mass to determine unit weight. If you are using digital scales or scales with springs for the force-measuring mechanism, use the "Pounds-Force" Tab.

Obtain Sample for Wet Density

- Excavate hole with large metal spoon or garden trowel that is 3 to 4" in diameter and 3 to 4 inches deep in order to obtain a sample of at least 500 grams (1.1 lbs).

- Fill the cone on the apparatus with the sand from the sandcone to determine the weight of the sand in the cone.

-Weight of sand from full cone (W₁) in Pounds (grams) = 1573.000 grams
-Weight of cone/jar apparatus with sand = (W) in grams 6384.000 grams
-Enter pre-determined density of sand in jar = 1.530 g/cm³

-Place base plate over excavated hole with hole in base plate in center of excavated hole.

-Weigh moist/wet soil from excavation W_s = 1430.000 grams

- Place cone (attached to sand jar) over hole and open valve.

- Turn Valve off and remove cone and base plate. If excavation is not full, spread sand evenly in excavation and repeat last step.

- Determine Volume of Hole Excavated.

- Weigh jar and remaining sand (W₂) 3795.000 grams + weight of sand in the full cone (W₁) and subtract from weight of full jar (W): W - (W₁+W₂) 1016.000 grams

Volume of Excavation = 664.052 cm³

Wet Density of Excavated Soil W_w = 2153.445 kg/m³

Dry Density of Soil

- Place moist soil obtained from the excavation into pan for drying after weighing on the scale

- Place material in iron skillet over active burner and break apart the core so that it can be dried over the heat.

- Carefully remove dried material from skillet making sure to transfer all material into a tin or other apparatus to be weighed.

- Obtain weight in grams of dry soil W_d = 1198.000 grams

- Moisture Content of sample = 16.224 %

Soil Dry Density = 1852.844 kg/m³

***Soil Dry Unit Wt. = 115.62 lb/ft³ *** measured in force (lb/ft³)



Report On: Daily Field Activity - Concrete

Lab No: 20142

Project No: 2024-1313-04T Cust No:1313

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Client: Civil & Environmental Consultants, Inc.

Report Date: 08/28/2024

Project: 13082 E Manito Rd., Pekin, IL

Test Date: 08/22/2024

Report No: 001

Tested By: Charles Tomas

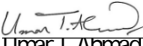
Location: Floawble Fill

NST representative was on site as scheduled and observed the flowable concrete fill placement. A total of 4 concrete loads were supplied by Roanoke, Pekin Plant . A total of 36 cubic yards of flowable concrete mix RSMFF03 were placed.

NST representative performed field concrete slump, air content and temperature tests on load 1. Slump, air content and temperature test results were consistent with the project specifications. A total of 1 set of 9 cylinders were cast as per project specifications from load 1 for laboratory curing and compressive strength testing. Test results are included on the Air Content and Slump report.

Remarks: No pictures taken.

Orig: Civil & Environmental Consultants, Inc.
Attn: Mr. Matthew D. Breitenbach
(1-ec copy)
1-cc Laboratory

Respectfully Submitted,

Umar T. Ahmad, PE





Report On: Air Content & Slump

Lab No: 20142-1

Project No: 2024-1313-04T Cust No:1313

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Client: Civil & Environmental Consultants, Inc.
Project: 13082 E Manito Rd., Pekin, IL
Report No: 002
Location: Flowable Fill
Material: PCC

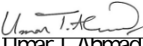
Report Date: 08/28/2024
Test Date: 08/22/2024
Sampled By: Charles Tomas

Spec Nbr	Sampled At	Truck No	Total Yards	Ticket No	Water Added	Mix Code	Time Sampled	Amb Temp (°F)	Mix Temp (°F)	Slump (In.)	Air %
1	Truck		90	2064652	No	RSMFF03	10:40 am	72	70	9.00	5.50
Placement Location: Flowable Fill											

Date Tested: 08/22/2024
Source: Roanoke Concrete
Plant: Pekin

Remarks:
Test Method (As Applicable): ASTM C143, C231, C172, C1064, C31

Orig: Civil & Environmental Consultants, Inc.
Attn: Mr. Matthew D. Breitenbach
(1-ec copy)
1-cc Laboratory

Respectfully Submitted,

Umar T. Ahmad, PE





Report On: Concrete Compression

Lab No: 20142-2

Project No: 2024-1313-04T Cust No:1313

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Client: Civil & Environmental Consultants, Inc.
Project: 13082 E Manito Rd., Pekin, IL
Report No: 003
Location: Flowable Fill

Report Date: 09/03/2024
Test Date: 08/22/2024
Sampled By: Charles Tomas

Table with columns: Cylinder Marked, Age Tested (date : days), Diameter (in), Area (in²), Max Load (lbs), Break Type, Cure Loc, Compressive Strength (PSI), Average Strength (PSI), Tested By. Rows A through I with various test results and 'HOLD' comments.

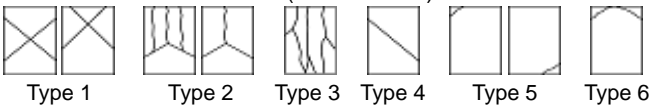


Table with columns: Measurement, Specification, Specification, Weather. Rows for Ambient, Mix, Slump, Air Content, Unit Weight, Source, Plant, Truck No, Mix Code, Ticket No, Sampled At, Transported By, Placement Date, Time Batched, Time Sampled, Curing Method.

Quantity Represented: 9 cu. yds. at 9 cu. yds. placed of a 36 cu. yd. total placement
Placement Location:
Sample Location: Flowable Fill
Remarks: CLSM
Test Method (As Applicable): Unless noted, concrete was sampled and tested in accordance with ASTM C172, C143, C231 or C173, C1054 and C138. Compressive strength tests per ASTM C39.

Orig: Civil & Environmental Consultants, Inc.
Attn: Mr. Matthew D. Breitenbach
(1-ec copy)
1-cc Laboratory

Respectfully Submitted,
Umar T. Ahmad, PE
Professional Engineer of Illinois