

WEEKLY SUMMARY REPORT

Week Ending: 08/24/2024 (Saturday)

Report No.: W-02-082424

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Civil & Environmental Consultants, Inc.

PROJECT INFORMATION

PROJECT NAME:	BYPASS BASIN RETROFIT CQA		
LOCATION:	Powerton Generating Station	CEC PROJECT NO:	343-014.0200
PLANS AND SPECS:	S&L Bypass Basin Retrofit Project	WEATHER:	Sunny, No Rain
ISSUED DATE:	07/12/24	TEMP. RANGE (°F)	52-90

PERSONNEL

FIELD REP(S):	Derek Dorsz, Alexander Bush, Saurabh Saawant	CEC PROJ. MANAGER(S):	Dean Jones
CLIENT:	Midwest Generation	CLIENT CONTACT(S):	Joe Kotas
CONTRACTOR:	Bluff City Construction (BCC)	SUPERVISOR:	Larry Hunt
CONTRACTOR:	Clean Air and Water Systems (CAAWS)	SUPERVISOR:	Andy Khamarlorm

SAFETY MEETINGS AND PARTICIPATION

Participation in Contractor's Tailgate Safety Meeting? Yes No Vehicle Check Performed? Yes or No
Plan for the Day Required (WSM 200.25): Yes (CLICK LINK BELOW) No

WORK PERFORMED SINCE CEC'S LAST VISIT⁽¹⁾

Work performed since CEC representative's last site visit? ⁽¹⁾ Yes No
Date CEC representative was last onsite: **Saturday, August 24, 2024**

⁽¹⁾ Critical work or work requiring continuous observation that has been completed without CEC representation being present onsite. CEC was not made aware that this work was being completed.

SUMMARY OF WORK OBSERVED, LOCATION, AND CONTRACTOR PERFORMING WORK

- Bluff City Construction (BCC) received loads of structural fill.
- Placed, graded, and compacted the final lift of structural fill within the bottom of the bypass basin and inlet structure, including adding water to achieve adequate compaction of the lifts.
 - Each lift thickness verified before and after compaction efforts to verify lift thicknesses.
 - Each lift compaction tested with the nuclear density gauge and sand cone, see attachments for reports.
- Graded structural fill on access ramp.
- Surveyed structural fill grades.
- Fusion welded 6" HDPE perforated leachate collection pipe. Perforations were orientated properly.
- Installed electrofusion couplers on 12" HDPE riser pipe.
- Installed non-woven geotextile, crushed aggregate bedding stone (CAB) within central trench, then installed 6" leachate collection pipe and 12" riser pipe within the trench and sump area.
- Excavated anchor trench on south, west, southeast, and northwest edges of basin.
- Placed lower sand cushion layer on top of the west and south sides of the basin.
- Received delivery of GCL.
 - Rolls unloaded and staged in the staging area located east of the bypass basin. Unloading methods and staging area were acceptable.
 - Rolls matched rolls listed in the approved submittals.
- Otto Baum applied Recrete 20 to inlet structure and grouted pipe at the outlet structure.
- Otto Baum filled vertical CMP with flowable fill.
- CAAWS inspected and approved of the bypass basin subgrade,
- CAAWS deployed and installed GCL and geomembrane across the basin.

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- Installation procedures from specifications were followed.
- CAAWS performed testing on seams of the geomembrane.
- CAAWS installed batten strips on outlet structure.
- CAAWS performed detail work around concrete structures and slope indicators.

UNEXPECTED, UNUSUAL, OR NONCONFORMING OBSERVATIONS (NEW / RESOLVED)

Unexpected, unusual, or nonconforming work observed? Yes No

- N/A

SUMMARY OF MEETINGS / DISCUSSIONS / PHONE CONVERSATIONS

- N/A

ATTACHMENTS

- Field Density Test Report and Sand Cone Test Reports
- Results of HDPE destructive testing.

DESCRIPTION OF SAMPLES TAKEN OR MATERIALS DELIVERED TO LAB

- PBBR-SF-5 sample collected and taken to lab.
- PBBR-CAB-1 sample collected and taken to lab.
- CAAWS removed two 2-foot sections from selected rolls of GCL for laboratory testing.
- Samples of flowable fill were collected and taken to lab.
- HDPE geomembrane destructive samples (DS-1 through DS-8) were collected, overnighted to lab, and received results.

PHOTOGRAPHS



Photo 1: shows storage of liner.

* No representations or warranties are made regarding the accuracy of the information generated by the Theodolite application, which is stamped on the photo, or the suitability of that information for any; legal, engineering, surveying, or other use or purpose.

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Photo 2: shows excavation of anchor trench.



Photo 3: shows deployment of GCL, geomembrane, and installation of CAB.

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Photo 4: shows batten strips along outlet structure.

13222 E Manito Rd, Pekin, IL 61554, USA

Latitude
40.54255756°

Longitude
-89.67607037°

Local 11:51:33
GMT 16:51:33

Altitude 141 m
Thursday, 22.08.2024

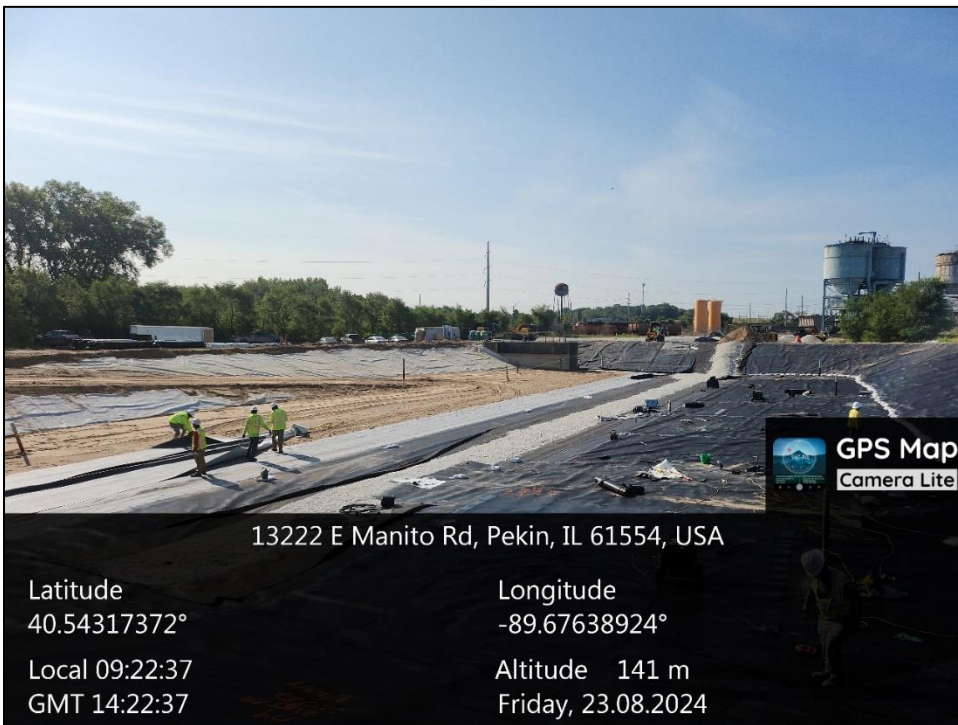


Photo 5: shows placement of geomembrane on western half of basin.

13222 E Manito Rd, Pekin, IL 61554, USA

Latitude
40.54317372°

Longitude
-89.67638924°

Local 09:22:37
GMT 14:22:37

Altitude 141 m
Friday, 23.08.2024

APPROVED BY

FIELD REP: Derek Dorsz DATE: 08/24/2024 CEC MANAGER: Dean Jones DATE: 08/26/2024

This document is draft until reviewed and approved by a Project Manager

NOTICE: Our firm's professionals are represented onsite solely to observe operations of the contractor identified to form opinions about the adequacy of those operations and to report those opinions to our client. The presence and activities of our field representative do not relieve the contractor from its obligation to meet contractual requirements. The contractor retains sole responsibility for site safety and the methods operations and sequences of construction.

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DAILY FIELD REPORT

Date: 08/20/2024 (Tuesday)

Report No.: 08-082024

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NUCLEAR COMPACTION TEST DATA (ASTM D6938)

PROJECT NAME: Powerton Bypass Basin Retrofit PROJECT NUMBER: 343-014
CEC TECHNICIAN: Derek Dorsz GAUGE NUMBER: 27636
CONTRACTOR: Bluff City Construction

STANDARD COUNTS:

DENSITY $\frac{954}{613}$ % DEVIATION $\frac{0.32}{0.33}$ ±1% PASSING
MOISTURE $\frac{954}{613}$ % DEVIATION $\frac{0.32}{0.33}$ ±2% PASSING

TEST NUMBER	13	14	15	16		
LOCATION	N: 1411934.6 E: 2432898.2	N: 1411910.6 E: 2432893.1	N: 1411886.5 E: 2432902.8	N: 1412025.9 E: 2432879.7		
ELEVATION OR LIFT NUMBER	Lift 4 Z: 458.5	Lift 4 Z: 458.2	Lift 4 Z: 458.3	Lift 4 Z: 458.4		
LIFT THICKNESS (in.)	8	8	8	8		
NUMBER OF PASSES	2	2	2	2		
PROBE DEPTH (in.)	0	0	0	0		
FIELD WET DENSITY (pcf)	113.3	116.2	109.8	113.7		
FIELD DRY DENSITY (pcf)	107.3	108.5	107.4	107.9		
COMPACTION (%)	95.1	96.2	95.2	95.7		
COMPACTION PASS/FAIL	PASS	PASS	PASS	PASS		
FIELD MOISTURE (%)	5.6	7.1	5.4	5.4		
MOISTURE PASS/FAIL	PASS	PASS	PASS	PASS		
LAB PROCTOR MDD	112.8	112.8	112.8	112.8		
PROCTOR TYPE (Mod./Stan.)	Modified	Modified	Modified	Modified		
SPECIFIED MIN. COMPACTION (%)	95.0	95.0	95.0	95.0		
LABORATORY OMC (%)	12.0	12.0	12.0	12.0		
SPECIFIED MOISTURE RANGE	N/A	N/A	N/A	N/A		
RETEST NUMBER	N/A	N/A	N/A	N/A		
REMARKS						

NOTES:

- MDD denotes Maximum Dry Density.
- OMC denotes Optimum Moisture Content.
- Elevations and lift thicknesses are approximate.
- N denote Northing, E denotes Easting, and Z denotes elevation. Coordinates given by Bluff City Construction.

Powerton Bypass Basin Retrofit
Sand Cone Test

Sand Cone Analysis (Cannot be performed in soils with coarse aggregates)

Date: 8/20/24 Test No: 16

This method is using balance scales measuring mass to determine unit weight. If you are using digital scales or scales with springs for the force-measuring mechanism, use the "Pounds-Force" Tab.

Obtain Sample for Wet Density

- Excavate hole with large metal spoon or garden trowel that is 3 to 4" in diameter and 3 to 4 inches deep in order to obtain a sample of at least 500 grams (1.1 lbs).

- Fill the cone on the apparatus with the sand from the sandcone to determine the weight of the sand in the cone.

-Weight of sand from full cone (W₁) in Pounds (grams) = 1573.000 grams
-Weight of cone/jar apparatus with sand = (W) in grams 7118.000 grams
-Enter pre-determined density of sand in jar = 1.530 g/cm³

-Place base plate over excavated hole with hole in base plate in center of excavated hole.

-Weigh moist/wet soil from excavation W_s = 597.000 grams

- Place cone (attached to sand jar) over hole and open valve.

- Turn Valve off and remove cone and base plate. If excavation is not full, spread sand evenly in excavation and repeat last step.

- Determine Volume of Hole Excavated.

- Weigh jar and remaining sand (W₂) 5047.000 grams + weight of sand in the full cone (W₁) and subtract from weight of full jar (W): W - (W₁+W₂) 498.000 grams

Volume of Excavation = 325.490 cm³

Wet Density of Excavated Soil W_w = 1834.157 kg/m³

Dry Density of Soil

- Place moist soil obtained from the excavation into pan for drying after weighing on the scale

- Place material in iron skillet over active burner and break apart the core so that it can be dried over the heat.

- Carefully remove dried material from skillet making sure to transfer all material into a tin or other apparatus to be weighed.

- Obtain weight in grams of dry soil W_d = 569.000 grams

- Moisture Content of sample = 4.690 %

Soil Dry Density = 1751.986 kg/m³

***Soil Dry Unit Wt. = 109.32 lb/ft³ *** measured in force (lb/ft³)



Date: 2024-08-24

Mail To:
Derek Dorsz
Civil and Environmental Consultants
1230 E Diehl Rd, Suite 200
Naperville , IL , 60563

Bill To:
Civil and Environmental Consultants
343-014

e-mail:
ddorsz@cecinc.com abush@cecinc.com djones@cecinc.com

Dear Dorsz,

Thank you for consulting with TRI/Environmental, Inc. (TRI) for your geosynthetics testing needs. TRI is pleased to submit this final report for laboratory testing.

Project: **Bypass Basin Retrofit Powerton**

TRI Job Reference Number: **81492**

Material(s) Tested: (6) Heat Fusion Weld Seam(s)
(2) Single Fusion Weld Seam(s)

Test(s) Requested: SAME DAY Peel and Shear
(ASTM D 6392/GRI GM19/D 4437/NSF 54/882 mod.)

Codes:	
AD	Adhesion Failure (100% Peel)
BRK	Break in sheeting away from Seam edge.
SE	Break in sheeting at edge of seam.
AD-BRK	Break in sheeting after some adhesion failure - partial peel.
SIP	Separation in the plane of the sheet (leaving the bond intact).
FTB	Film tearing bond (all non "AD" failures).
NON-FTB	100% peel.

If you have any questions or require any additional information, please call us at 1-800-880-8378.

Sincerely,

Mansukh Patel
Sr. Laboratory Coordinator
Geosynthetic Services Division
<http://www.geosyntheticstestinc.com>



DESTRUCTIVE SEAM QUALITY ASSURANCE TEST RESULTS

TRI Client: Civil and Environmental Consultants

Project: Bypass Basin Retrofit Powerton

Material: 60 mil. HDPE

SAME DAY Peel and Shear (ASTM D 6392/GRI GM19/D 4437/NSF 54/882 mod.)

TRI Log#: 81492

TEST REPLICATE NUMBER

Table with 7 columns: PARAMETER, 1, 2, 3, 4, 5, MEAN

Sample ID: DS-1 | Weld: Heat Fusion

Main data table for Sample ID DS-1, including Side A, Side B, and Shear results.

Sample ID: DS-2 | Weld: Heat Fusion

Main data table for Sample ID DS-2, including Side A, Side B, and Shear results.

The testing herein is based upon accepted industry practice as well as the test method listed. Test results reported herein do not apply to samples other than those tested. TRI neither accepts responsibility for nor makes claim as to the final use and purpose of the material. TRI observes and maintains client confidentiality. TRI limits reproduction of this report, except in full, without prior approval of TRI.



DESTRUCTIVE SEAM QUALITY ASSURANCE TEST RESULTS

TRI Client: Civil and Environmental Consultants

Project: Bypass Basin Retrofit Powerton

Material: 60 mil. HDPE

SAME DAY Peel and Shear (ASTM D 6392/GRI GM19/D 4437/NSF 54/882 mod.)

TRI Log#: 81492

TEST REPLICATE NUMBER

Table with 7 columns: PARAMETER, 1, 2, 3, 4, 5, MEAN

Sample ID: DS-3 | Weld: Heat Fusion

Main data table for Sample DS-3, including Side A, Side B, and Shear results with mean values in boxes.

Sample ID: DS-4 | Weld: Heat Fusion

Main data table for Sample DS-4, including Side A, Side B, and Shear results with mean values in boxes.

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DESTRUCTIVE SEAM QUALITY ASSURANCE TEST RESULTS

TRI Client: Civil and Environmental Consultants

Project: Bypass Basin Retrofit Powerton

Material: 60 mil. HDPE

SAME DAY Peel and Shear (ASTM D 6392/GRI GM19/D 4437/NSF 54/882 mod.)

TRI Log#: 81492

TEST REPLICATE NUMBER

Table with 7 columns: PARAMETER, 1, 2, 3, 4, 5, MEAN. Rows include Sample ID: DS-7 | Weld: Heat Fusion, Side: A (Peel Strength, Incursion, Locus, NSF), Side: B (Peel Strength, Incursion, Locus, NSF), and Shear (Strength, Elongation).

Table with 7 columns: PARAMETER, 1, 2, 3, 4, 5, MEAN. Rows include Sample ID: DS-8 | Weld: Heat Fusion, Side: A (Peel Strength, Incursion, Locus, NSF), Side: B (Peel Strength, Incursion, Locus, NSF), and Shear (Strength, Elongation).

The testing herein is based upon accepted industry practice as well as the test method listed. Test results reported herein do not apply to samples other than those tested. TRI neither accepts responsibility for nor makes claim as to the final use and purpose of the material. TRI observes and maintains client confidentiality. TRI limits reproduction of this report, except in full, without prior approval of TRI.



DESTRUCTIVE SEAM QUALITY ASSURANCE TEST RESULTS - SINGLE TRACK

TRI Client: Civil and Environmental Consultants

Project: Bypass Basin Retrofit Powerton

Material: 60 mil. HDPE

SAME DAY Peel and Shear (ASTM D 6392/GRI GM19/D 4437/NSF 54/882 mod.)

TRI Log#: 81492

PARAMETER	TEST REPLICATE NUMBER					MEAN
	1	2	3	4	5	
Sample ID: DS-5 Weld: Single Extrusion						
Side: Peel						Peel
Peel Strength (ppi)	141	142	113	100	109	121
Peel Incursion (%)	<5	<5	<5	<5	<5	
Peel Locus Of Failure Code	SE	SE	SE	SE	SE	
Peel NSF Failure Code	FTB	FTB	FTB	FTB	FTB	
Shear						Shear
Shear Strength (ppi)	179	179	179	186	180	181
Shear Elongation @ Break (%)	>50	>50	>50	>50	>50	
Sample ID: DS-6 Weld: Single Extrusion						
Side: Peel						Peel
Peel Strength (ppi)	155	158	146	143	148	150
Peel Incursion (%)	<5	<5	<5	<5	<5	
Peel Locus Of Failure Code	SE	SE	SE	SE	SE	
Peel NSF Failure Code	FTB	FTB	FTB	FTB	FTB	
Shear						Shear
Shear Strength (ppi)	174	169	175	175	169	172
Shear Elongation @ Break (%)	>50	>50	>50	>50	>50	

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