NRG – Indian River Landfill Run-on and Run-Off Control System Plan

Indian River Landfill Dagsboro, DE

SCS ENGINEERS

25221174.00 | August 2021

St. Charles, IL

Table of Contents

Sect	ction Page							
Profe	Professional Engineer Certification1							
1.0	Introduction	2						
2.0	2.0 Regulatory Requirements							
3.0	Previous Run-On and Run-Off Control Systems Plans							
4.0	4.0 2021 Run-On and Run-Off Control system Plan update							
	4.1 Run-On control System	3						
	4.2 Run-Off control System	3						
	4.3 Hydrologic and Hydraulic Analysis	4						
	4.4 Results and Conclusions	4						
5.0	Certifications	5						
6.0	Recordkeeping and Periodic Updates5							
7.0	Landfill Development							
8.0	Erosion and Sediment Controls							
9.0	Inspection and Maintenance	6						
10.0) References	6						

Appendix A

Appendix A.1	Rainfall Totals and Distribution
Appendix A.2	Subcatchment Delineation
Appendix A.3	Weighted Curve Number Determination
Appendix A.4	Time of Concentration Calculation
Appendix A.5	Subcatchment Discharge Rates
Appendix A.6	Landfill Conveyance Feature Sizing
Appendix A.7	Detention Basin Sizing
Appendix A.8	HydroCAD Output Files

PROFESSIONAL ENGINEER CERTIFICATION

I, Richard D. Southorn, hereby certify that this Run-On and Run-Off Control System Plan meets the requirements of 40 CFR §257.81(c), was prepared by me or under my direct supervision, and that I am a duly licensed Professional Engineer under the laws of the State of Delaware.

This plan has been prepared as a periodic update to the initial Run-On and Run-Off Control System Plan that was certified on October 12, 2016.

Richard Southorn, PE License 20894 Expires 6/30/2022

1.0 INTRODUCTION

The Indian River Landfill, located in Dagsboro, DE, has been permitted by the Delaware Department of Natural Resources and Environmental Control (DNREC) for the purpose of disposing coal combustion residuals (CCR) that is generated by the Indian River Generating Station. This Run-on and Run-off Control System Plan documents that the Landfill's run-on and run-off control systems have been designed and constructed to meet the applicable requirements of Title 40 Code of Federal Regulations (CFR) §257.81 of the CCR Rule.

2.0 **REGULATORY REQUIREMENTS**

40 CFR §257.81 Run-on and run-off controls for CCR landfills.

- (a) The owner or operator of an existing or new CCR landfill or any lateral expansion of a CCR landfill must design, construct, operate, and maintain:
 - (1) A run-on control system to prevent flow onto the active portion of the CCR unit during the peak discharge from a 24-hour, 25-year storm; and
 - (2) A run-off control system from the active portion of the CCR unit to collect and control at least the water volume resulting from a 24-hour, 25-year storm.
- (b) Run-off from the active portion of the CCR unit must be handled in accordance with the surface water requirements under 40 CFR §257.3-3.
- (c) Run-on and run-off control system plan
 - (1) Content of the plan. The owner or operator must prepare initial and periodic run-on and run-off control system plans for the CCR unit according to the timeframes specified in paragraphs (c)(3) and (4) of this section. These plans must document how the run-on and run-off control systems have been designed and constructed to meet the applicable requirements of this section. Each plan must be supported by appropriate engineering calculations. The owner or operator has completed the initial run-on and run-off control system plan when the plan has been placed in the facility's operating record as required by 40 CFR §257.105(g)(3).

With reference to 40 CFR §257.81(c) above, the Initial Run-On and Run-Off Control System Plan (RORO Plan) was required to be developed no later than October 17, 2016 for existing landfills (40 CFR §257.81(c)(3)(i)). Updates to the RORO Plan are required whenever there is a change in conditions that would substantially affect the written plan in effect (40 CFR §257.81(2), or within five years of the previous plan (40 CFR §257.81(c)(4)).

The owner or operator must obtain a certification from a qualified professional engineer or approval from the Participating State Director or approval from EPA where EPA is the permitting authority stating that the initial and periodic RORO Plans meet the requirements of 40 CFR §257.81.

3.0 PREVIOUS RUN-ON AND RUN-OFF CONTROL SYSTEMS PLANS

The initial RORO Plan for Indian River Landfill was completed on October 12, 2016 and has been placed on a publicly available website (<u>https://www.nrg.com/legal/coal-combustion-residuals.html</u>) since 2016. As previously stated in Section 2.0, updates to the RORO Plan are required whenever there is a change in conditions that would substantially affect the written plan in effect (40 CFR §257.81(2), or within five years of the previous plan (40 CFR §257.81(c)(4)). No amendments or periodic updates have been completed prior to this Periodic RORO Plan update.

4.0 2021 RUN-ON AND RUN-OFF CONTROL SYSTEM PLAN UPDATE

This document has been prepared as the five-year periodic update to the initial RORO Plan. The runon and run-off design and conveyance strategy described in this plan is consistent with the initial RORO Plan. However, this periodic RORO Plan expands on the previous evaluation by reporting the hydrologic modeling results for all conveyance elements utilized at the landfill. This plan will replace the initial RORO Plan on a forward-going basis.

The run and run-off control conveyance features at the Indian River Landfill have been reviewed as part of this 2021 Periodic RORO Plan update and have been found to meet the following requirements:

40 CFR §257.81(a) "The owner or operator of an existing or new CCR landfill or any lateral expansion of a CCR landfill must design, construct, operate, and maintain:

- (1) A run-on control system to prevent flow onto the active portion of the CCR unit during the peak discharge from a 24-hour, 25-year storm.
- (2) A run-off control system from the active portion of the CCR unit to collect and control at least the water volume resulting from a 24-hour, 25-year storm."

Certification from a professional engineer that the run-on and run-off conveyance features are appropriate is provided on page 1 of this report. Conveyance features that comprise the run-on and run-off control systems at the Indian River Landfill are depicted in **Appendix A**.

4.1 RUN-ON CONTROL SYSTEM

The permitted Indian River Landfill is bound by a perimeter berm structure that is approximately 8 to 20 feet in height. The perimeter berm structure has been constructed to prevent stormwater run-on onto active and closed portions of the Landfill.

4.2 RUN-OFF CONTROL SYSTEM

Stormwater falling on the Landfill is managed by terrace berms, letdown pipes, perimeter drainage ditches, and detention basins. Terrace berms have been designed to intercept runoff from the final landform in order to prevent erosion and reduce the drainage length of runoff. The terrace berms are generally spaced every 20 vertical feet, or every 60 slope feet. Terrace berms direct stormwater towards letdown pipes that convey stormwater to the perimeter drainage ditches. Letdown pipes are 24" diameter, high-density polyethylene pipe with smooth interior walls that convey stormwater down to the perimeter ditches. Stormwater collected in the perimeter ditches are conveyed to either the Northeast Basin system or the Southwest Basin system. The Northeast Basin system consists of a

West and East Forebay designed to facilitate sediment knock-out prior to entry into the Northeast Basin. The Southwest Basin system includes the Southwest Forebay that is also designed to facilitate sediment knock-out prior to entry into the Southwest Basin.

Stormwater collected from the Indian River Landfill is permitted to discharge into Island Creek, located north of the Indian River Landfill, through a National Pollution Discharge Elimination System (NPDES) permitted outfall. In accordance with 40 CFR §257.81(b), this is consistent with the surface water requirements under 40 CFR §257.3-3¹.

4.3 HYDROLOGIC AND HYDRAULIC ANALYSIS

Engineering calculations to evaluate the run-on and run-off control system at the Indian River Landfill consisted of a hydrologic and hydraulic (H&H) stormwater model prepared using HydroCAD stormwater modeling software. Information used to prepare the HydroCAD stormwater model are summarized within **Appendix A**.

H&H stormwater analyses has been prepared for the 25-year, 24-hour storm event in accordance with the CCR Rule. All stormwater management calculations were performed utilizing methods outlined in the Soil Conservation Services (SCS) publication on Urban Hydrology for Small Watersheds, Technical Release 55 (TR-55) and computed by HydroCAD software version 10.00. Site conditions were modeled to reflect stormwater runoff based on runoff characteristics such as land cover, soil type, and time of concentration. Stormwater management features such as terrace berms, letdown pipes, perimeter channels, and detention basins have also been modeled.

Detention basins (not included forebays) have been modeled with two discharge methods. Discharge pipes and spillway systems that are designed to manage the flow out of each detention basin have been modeled. Additionally, exfiltration through native soils has also been modeled as a discharge method. According to the USDA Natural Resources Conservation Services (NRCS) Web Soil Survey of Sussex County, Delaware (NRCS Soil Survey)², the rate of infiltration for the native soils is 5.95 in/hr. In accordance with the Sussex Conservation District guidelines, a rate of one-half the posted rate was used for design purposes; therefore, the modeled rate was 2.97 in/hr for existing and proposed conditions in all detention basins. Each detention basin has been modeled with these two discharge methods to provide an accurate depiction of the conditions that are observed at the Indian River Landfill.

4.4 **RESULTS AND CONCLUSIONS**

The HydroCAD stormwater model of the Indian River Landfill was developed to evaluate whether the peak flow from the 25-year, 24-hour storm event could be accommodated without overtopping the run-on and run-off control systems.

The run-on control system discussed previously is designed and constructed to divert stormwater away from the Indian River Landfill and meets the requirements of 40 CFR §257.81(a)(1).

The run-off control system is designed to collect and convey stormwater from the Indian River Landfill and discharge through the permitted outfalls. Based on the results of the HydroCAD stormwater model, the run-off control system was found to accommodate the 25-year, 24-hour storm event without overtopping³. The run-on control system meets the requirements of 40 CFR §257.81(a)(2).

5.0 CERTIFICATIONS

Richard D. Southorn, a licensed Professional Engineer in the State of Delaware, has overseen the preparation of this Run-On and Run-Off Control System Plan. A certification statement in accordance with 40 CFR $\S257.81(c)(5)$ is provided on **Page 1** of this plan.

6.0 **RECORDKEEPING AND PERIODIC UPDATES**

This Run-On and Run-Off Control System Plan, and all periodic plans, will be placed in the facility's operating record and on NRG's CCR Rule Compliance Data and Information website, as will all amendments. Periodic Run-On and Run-Off Control System Plans will be completed every 5 years per 40 CFR §257.81(c)(4).

Notification will be provided when this Run-On and Run-Off Control System Plan, and all periodic plans, are available in the facility's operating record and on the facility's website in accordance with 40 CFR §257.105(g), §257.106(g), and §257.107(g).

7.0 LANDFILL DEVELOPMENT

Construction activities will include installation of sediment and stormwater controls, waste placement and compaction, and final cover construction.

During cell construction and filling, additional temporary measures will be incorporated to divert stormwater away from active landfilling and liner construction areas. These temporary measures will intercept the runoff from undisturbed areas before it reaches the construction areas (disturbed areas), and the runoff will be conveyed to the landfill perimeter as practical. Any stormwater that collects within the landfill excavation will be routed to temporary stormwater collection sumps. Similarly, any rainfall which ponds on the liner and leachate collection system prior to the placement of waste will be pumped into the stormwater management system.

Temporary diversion measures will be constructed around the active landfilling areas to the extent practical in order to divert stormwater from adjacent daily, intermediate and final cover slopes before it contacts any waste, thereby preventing it from coming into contact with the waste.

8.0 EROSION AND SEDIMENT CONTROLS

Prior to construction of Phase II, a Sediment and Stormwater Management Plan was submitted to and approved by the Delaware Department of Natural Resources and Environmental Control (DNREC). Temporary erosion and sediment controls were designed and implemented in accordance with the DNREC-approved Sediment and Stormwater Management Plan. In addition, the Indian River Generating Station maintains an NPDES Permit the covers the Indian River Landfill.

Erosion and sediment control techniques installed during construction of the Indian River Landfill will not only minimize sediment erosion but will improve the water quality of stormwater runoff. The following erosion and sediment controls will be utilized at the proposed Phase III expansion:

Barrier filters such as silt fence, compost filter logs, and straw bale barriers will be installed at a minimum along the entire length of all disturbed slopes where stormwater is being directly discharged off-site until permanent vegetation has been established and sediment control is no longer necessary. Barrier filters will also be used within stormwater drainage channels to provide additional sediment filtration.

- Vegetative filters provide biological filtration to improve water quality where concentrations of sediment are high and flow velocities are relatively low. Vegetative filters may be used along drainage ways or property lines. Vegetative filters may also be used on the side slopes of the detention basin to filter sediment from overland flow.
- Temporary seeding and/or stabilization matting will be installed on areas of exposed soils to minimize erosion.

9.0 INSPECTION AND MAINTENANCE

All temporary and permanent erosion and sediment control measures are maintained and repaired as needed to assure continued performance of their intended function. This program will include performance checks of facilities and grades, remedial grading, sediment removal, vegetative care, and maintenance. Inspections will address points of scour, slope failure, breaching or settling. Inspections are performed at an appropriate frequency in compliance with the Facility NDPES permit and Solid Waste permits. Maintenance includes clearing of sediment from barriers and the basins. Sediments are dredged from the sediment basins as necessary to maintain adequate stormwater detention and functionality of the outlet structures. Sediment removed from the barriers and the detention basins will not be placed in floodplain areas or in areas without adequate BMPs in-place. As necessary, runoff collection features are cleaned, regraded, relined, rip-rapped, etc., to restore design capacities and correct problem areas. A written record of all inspections and maintenance is prepared and placed in the facility Stormwater Pollution Prevention Plan (SWPPP), which is kept at the site.

10.0 REFERENCES

- 1. U.S. Environmental Protection Agency, Standards for the Disposal of Coal Combustion Residuals in Landfills and Surface Impoundments, Title 40 Code of Federal Regulations Part §257. Federal Register 80, Subpart D, dated April 17, 2015.
- 2. USDA Natural Resources Conservation Service, Web Soil Survey, dated 2021.
- 3. SCS Engineers, Indian River Landfill– Stormwater Calculations, Run-On and Run-Off Control System Plan, dated 2021 (Provided as **Appendix A**).

[This page left blank intentionally]

APPENDIX A

- Appendix A.1 Appendix A.2 Appendix A.3 Appendix A.4 Appendix A.5 Appendix A.6 Appendix A.7 Appendix A.8
- Rainfall Totals and Distribution Subcatchment Delineation Weighted Curve Number Determination Time of Concentration Calculation Subcatchment Discharge Rates Landfill Conveyance Feature Sizing Detention Basin Sizing HydroCAD Output Files

Appendix A.1

Rainfall Totals and Distributions

S C S	ENGINEERS	SHEET NO.		1 of	
Job No.	25221174	CALC. NO.		1	
Job:	Indian River Landfill – Run-on / Run-off Evaluation	REV. NO.		1	
Client	NRG	BY	SJL	DATE	7/10/2021
Subject	Rainfall Totals	CHK'D.	RDS	DATE	7/30/2021

Problem Statement

Determine the rainfall totals and distributions of the 25-year, 24-hour storm event. The rainfall totals and distributions are used in the HydroCAD Version 10 (HydroCAD) computer model to determine rainfall runoff quantities.

<u>Given</u>

Stormwater discharge rates for the 25-year, 24-hour storm are used to determine the adequacy of the Landfill stormwater management features based on the proposed closure plan conditions.

Rainfall data for the 25-year, 24-hour storm event was obtained from the State of Delaware Department of Natural Resources and Environmental Control (DNREC) Regulatory Guidance Memorandum RGM-1. This document identifies the use of National Oceanic & Atmosphere Administration (NOAA) rainfall distribution curves found in Atlas 14 to be appropriate for stormwater evaluations in Delaware. This document is attached and identifies a specific rainfall depth for the county where the Landfill resides (Sussex County). An excerpt of this document is attached.

Technical Release 55 (TR-55) was consulted to determine the appropriate storm distribution pattern. TR-55 was prepared by the United States Department of Agriculture - Natural Resources Conservation Service (USDA-NRCS) to provide guidance for urban hydrology for small watersheds. According to TR-55, the Type II 24-hour storm distribution is appropriate for the Landfill, which resides in Sussex County, Delaware. This storm distribution has been programmed into HydroCAD by the software manufacturer and can be chosen from a drop-down list of distribution patterns. A copy of the rainfall distribution map is attached.

<u>Results</u>

The following table provides the rainfall depths provided in the DNREC RGM-1 document.

Table 1 – Rainfall Depths					
Storm Event	Interpolated Rainfall Depth (in)				
25-year, 24-hour	6.68				

The Type II 24-hour storm distribution will be chosen from the drop-down list of distribution patterns programmed into HydroCAD.



STATE OF DELAWARE DEPARTMENT OF NATURAL RESOURCES AND ENVIRONMENTAL CONTROL DIVISION OF WATERSHED STEWARDSHIP

OFFICE OF THE DIRECTOR

89 Kings Highway DOVER, DELAWARE 19901 PHONE: (302) 739-9921 FAX: (302) 739-6724

SEDIMENT & STORMWATER PROGRAM

REGULATORY GUIDANCE MEMORANDUM

RGM-1

Date: November 7, 2019

Title: Adoption of NOAA Rainfall Distribution Curves

Synopsis: The Delaware State Office of the Natural Resources Conservation Service (NRCS) has adopted the current National Oceanic & Atmospheric Administration (NOAA) Rainfall Distribution curves for the hydrologic design of conservation practices in Delaware in place of the NRCS Type II Rainfall Distribution Curves. In keeping with this, the Delaware Sediment & Stormwater Program will begin accepting the use of the NOAA Rainfall Distribution Curves for the hydrologic design of stormwater management practices intended to comply with the Delaware Sediment & Stormwater Regulations effective January 1, 2020. NOAA Curve C should be used in New Castle and Kent Counties. NOAA Curve D should be used for Sussex County.

Effective Date: January 1, 2020

Responsible Staff Member

andel K Steer

Randell K Greer, P.E. Engineer VI

Delaware's good nature depends on you!





Delaware Regional Rainfall Distributions

🏷 EFH-2 I	Estimating R	unoff and Peak Discl	harge				- 7 🗙
File Edit Vi	'iew Tools He	elp					
New	Ctrl+N	🗎 陆 H65 🢡					
Open		stion	Basic data		Rainfall/Discharge data	<u></u>	RCN
Recalculate	e	Bainfall - Type:				,	
Save	Ctrl+S						
Print	Ctrl+P	Frequency (yrs)	24-HR Rain	Peak Flow	Runoff		
Exit			(in)	(crs)	linj		
	Storm #1	1	2.71	9	1.15		
	Storm #2	2	3.30	13	1.62		
	Storm #3	5	4.29	21	2.45		
	Storm #4	10	5.16	27	3.22		
	Storm #5	25	6.46	37	4.41		
	Storm #6	50	7.61	47	5.49		
	Storm #7	100	8.90	57	6.72		
🕂 stari		nbox - Microsoft Out	EFH2 Supplement_Dr	S FFH-2 Estim	ating Ru	2 -	月前命9.55 3 50 8:11 AM

To complete the project, click File and Save. Print output if desired. Close EFH-2.

Appendix 1. County rainfall database (county.DE) Notes: Rainfall distribution for each county and zone are shown with the county name (Region C or D). The 24-hour rainfall duration values are in units of inches.

County/Zone	1-year	2-year	5-year	10-year	25-year	50-year	100-year
New Castle NOAA-C	2.64	3.20	4.09	4.85	5.99	6.97	8.05
Kent NOAA-C	2.71	3.30	4.29	5.16	6.46	7.61	8.90
Sussex NOAA-D	2.81	3.42	4.44	5.33	6.68	7.87	9.20



United States Department of Agriculture

Natural Resources Conservation Service

Conservation Engineering Division

Technical Release 55

June 1986

Urban Hydrology for Small Watersheds

TR-55

Appendix B

Synthetic Rainfall Distributions and Rainfall Data Sources

The highest peak discharges from small watersheds in the United States are usually caused by intense, brief rainfalls that may occur as distinct events or as part of a longer storm. These intense rainstorms do not usually extended over a large area and intensities vary greatly. One common practice in rainfall-runoff analysis is to develop a synthetic rainfall distribution to use in lieu of actual storm events. This distribution includes maximum rainfall intensities for the selected design frequency arranged in a sequence that is critical for producing peak runoff.

Synthetic rainfall distributions

The length of the most intense rainfall period contributing to the peak runoff rate is related to the time of concentration (T_c) for the watershed. In a hydrograph created with NRCS procedures, the duration of rainfall that directly contributes to the peak is about 170 percent of the T_c . For example, the most intense 8.5-minute rainfall period would contribute to the peak discharge for a watershed with a T_c of 5 minutes. The most intense 8.5-hour period would contribute to the peak for a watershed with a 5-hour T_c .

Different rainfall distributions can be developed for each of these watersheds to emphasize the critical rainfall duration for the peak discharges. However, to avoid the use of a different set of rainfall intensities for each drainage area size, a set of synthetic rainfall distributions having "nested" rainfall intensities was developed. The set "maximizes" the rainfall intensities by incorporating selected short duration intensities within those needed for longer durations at the same probability level.

For the size of the drainage areas for which NRCS usually provides assistance, a storm period of 24 hours was chosen the synthetic rainfall distributions. The 24hour storm, while longer than that needed to determine peaks for these drainage areas, is appropriate for determining runoff volumes. Therefore, a single storm duration and associated synthetic rainfall distribution can be used to represent not only the peak discharges but also the runoff volumes for a range of drainage area sizes.

Figure B-1 SCS 24-hour rainfall distributions



The intensity of rainfall varies considerably during a storm as well as geographic regions. To represent various regions of the United States, NRCS developed four synthetic 24-hour rainfall distributions (I, IA, II, and III) from available National Weather Service (NWS) duration-frequency data (Hershfield 1061; Frederick et al., 1977) or local storm data. Type IA is the least intense and type II the most intense short duration rainfall. The four distributions are shown in figure B-1, and figure B-2 shows their approximate geographic boundaries.

Types I and IA represent the Pacific maritime climate with wet winters and dry summers. Type III represents Gulf of Mexico and Atlantic coastal areas where tropical storms bring large 24-hour rainfall amounts. Type II represents the rest of the country. For more precise distribution boundaries in a state having more than one type, contact the NRCS State Conservation Engineer.





Rainfall data sources

This section lists the most current 24-hour rainfall data published by the National Weather Service (NWS) for various parts of the country. Because NWS Technical Paper 40 (TP-40) is out of print, the 24-hour rainfall maps for areas east of the 105th meridian are included here as figures B-3 through B-8. For the area generally west of the 105th meridian, TP-40 has been superseded by NOAA Atlas 2, the Precipitation-Frequency Atlas of the Western United States, published by the National Ocean and Atmospheric Administration.

East of 105th meridian

Hershfield, D.M. 1961. Rainfall frequency atlas of the United States for durations from 30 minutes to 24 hours and return periods from 1 to 100 years. U.S. Dept. Commerce, Weather Bur. Tech. Pap. No. 40. Washington, DC. 155 p.

West of 105th meridian

Miller, J.F., R.H. Frederick, and R.J. Tracey. 1973. Precipitation-frequency atlas of the Western United States. Vol. I Montana; Vol. II, Wyoming; Vol III, Colorado; Vol. IV, New Mexico; Vol V, Idaho; Vol. VI, Utah; Vol. VII, Nevada; Vol. VIII, Arizona; Vol. IX, Washington; Vol. X, Oregon; Vol. XI, California. U.S. Dept. of Commerce, National Weather Service, NOAA Atlas 2. Silver Spring, MD.

Alaska

Miller, John F. 1963. Probable maximum precipitation and rainfall-frequency data for Alaska for areas to 400 square miles, durations to 24 hours and return periods from 1 to 100 years. U.S. Dept. of Commerce, Weather Bur. Tech. Pap. No. 47. Washington, DC. 69 p.

Hawaii

Weather Bureau. 1962. Rainfall-frequency atlas of the Hawaiian Islands for areas to 200 square miles, durations to 24 hours and return periods from 1 to 100 years. U.S. Dept. Commerce, Weather Bur. Tech. Pap. No. 43. Washington, DC. 60 p.

Puerto Rico and Virgin Islands

Weather Bureau. 1961. Generalized estimates of probable maximum precipitation and rainfall-frequency data for Puerto Rico and Virgin Islands for areas to 400 square miles, durations to 24 hours, and return periods from 1 to 100 years. U.S. Dept. Commerce, Weather Bur. Tech. Pap. No. 42. Washington, DC. 94 P. Appendix A.2

Subcatchment Delineation

	SCS	ENGINEERS
--	-----	-----------

		SHEELNO.		I OT	
Job No.	25221174	CALC. NO.		2	
Job:	Indian River Landfill – Run-on/Run-off Evaluation	REV. NO.		1	
Client	NRG	BY	SJL	DATE	7/10/2021
Subject	Subcatchment Delineation	CHK'D.	RDS	DATE	7/30/2021

Problem Statement

Delineate the subcatchment areas (watersheds) for the proposed closure plan conditions for the Landfill.

<u>Given</u>

The stormwater management system that is described in this calculation represents all areas that will convey stormwater associated with the proposed closure plan for the Landfill. Subcatchment areas within the Landfill footprint were based on final closed conditions and existing/proposed stormwater management features. Subcatchment areas were delineated using topographic divides and stormwater management feature locations including terrace berms, letdown pipes, perimeter ditches, and culverts.

The subcatchment areas for the proposed conditions are shown on Figure A.2-1.

Results

Figure A.2-1 depicts the delineation of the stormwater subcatchments for the proposed closure plan for the Landfill. The attached **Table A.2-1** summarizes the approximate acreage of all subcatchment areas and identifies the ultimate discharge location for each area. The nomenclature for each subcatchment area is described below:

<u>Stormwater Management Feature Location (First Identifier)</u> SC = Subcatchment

<u>Sub-grouping (Second Identifier)</u> Letter = Sub-grouping identifier Number = Quantity within sub-group

NRG - Indian River Landfill

						Tabl Subcatchment A	e A.2-1 rea Summary Table						
6.3	3 Acres	12.0	3 Acres	3.05	5 Acres	5.29	Acres	7.65 Acres			Acres	4.78 Acres	
Northeast	t Basin (DP-1)	Northeast	Basin (DP-1)	Northeast Basin (DP-1)		Northeast Basin (DP-1)		Southwest Basin (DP-2)		Southwest Basin (DP-2)		Northeast Basin (DP-1)	
Subcatchment Identifier	Area (Acres)	Subcatchment Identifier	Area (Acres)	Identifier	Area (Acres)	Identifier	Area (Acres)	Subcatchment Identifier	Area (Acres)	Subcatchment Identifier	Area (Acres)	Subcatchment Identifier	Area (Acres)
SC-A1	0.74	SC-B1	0.43	SC-C1	0.42	SC-D1	1.78	SC-E1	5.74	SC-F1	0.97	SC-G1	0.59
SC-A2	0.39	SC-B2	0.89	SC-C2	0.04	SC-D2	0.10	SC-E2	0.15	SC-F2	1.22	SC-G2	1.10
SC-A3	0.81	SC-B3	3.16	SC-C3	0.55	SC-D3	1.13	SC-E3	0.31	SC-F3	1.36	SC-G3	0.43
SC-A4	0.80	SC-B4	1.18	SC-C4	0.31	SC-D4	1.01	SC-E4	0.39	SC-F4	0.85	SC-G4	0.40
SC-A5	0.43	SC-B5	1.27	SC-C5	0.52	SC-D5	1.05	SC-E5	0.46	SC-F5	0.43	SC-G5	0.44
SC-A6	0.46	SC-B6	0.03	SC-C6	0.31	SC-D6	0.14	SC-E6	0.27			SC-G6	0.40
SC-A7	0.27	SC-B7	0.46	SC-C7	0.48	SC-D7	0.07	SC-E7	0.22			SC-G7	0.22
SC-A8	0.50	SC-B8	0.53	SC-C8	0.30			SC-E8	0.12			SC-G8	0.10
SC-A9	0.68	SC-B9	0.13	SC-C9	0.06							SC-G9	0.81
SC-A10	0.37	SC-B10	0.48	SC-C10	0.05							SC-G10	0.27
SC-A11	0.72	SC-B11	0.02										
SC-A12	0.17	SC-B12	1.00										
		SC-B13	0.28										
		SC-B14	1.11										
		SC-B15	0.59										
		SC-B16	0.33										
		SC-B17	0.16										
6.44 Ultimate Disc Northeast	8 Acres charge Location: t Basin (DP-1)	3.2 Ultimate Disc Northeast	5 Acres charge Location: Basin (DP-1)	4.3 Ultimate Disc Northeast	Acres harge Location: Basin (DP-1)	11.3 Ultimate Disc Northeast	5 Acres charge Location: Basin (DP-1)	9.8 Ultimate Disc Northeast	Acres charge Location: Basin (DP-1)	4.2: Ultimate Disc Southwest	L Acres harge Location: Basin (DP-2)		
Subcatchment Identifier	Area (Acres)	Subcatchment Identifier	Area (Acres)	Subcatchment Identifier	Area (Acres)	Subcatchment Identifier	Area (Acres)	Subcatchment Identifier	Area (Acres)	Subcatchment Identifier	Area (Acres)		
SC-H1	0.54	SC-I1	1.91	SC-J1	2.81	SC-K1	2.80	SC-NE1	0.32	SC-SW1	0.17	Subcatchment Nome	anclature Kev
SC-H2	0.78	SC-12	0.54	SC-J2	0.14	SC-K2	1.40	SC-NE2	0.50	SC-SW2	0.40	Stormwater Manageme	ent Feature Location
SC-H3	0.82	SC-13	0.18	SC-J3	0.29	SC-K3	0.37	SC-NE3	0.47	SC-SW3	0.45	SC = Subcatchment	
SC-H4	0.71	SC-14	0.06	SC-J4	0.40	SC-K4	0.66	SC-NE4	0.46	SC-SW4	3.20	Sub-grouping (Second Letter = Sub-grouping i	<u>ldentifier)</u> dentifier
SC-H5	0.98	SC-15	0.40	SC-J5	0.42	SC-K5	0.69	SC-NE5	8.05			Number = Quantity with	hin sub-group
SC-H6	0.93	SC-16	0.16	SC-J6	0.24	SC-K6	1.11						
SC-H7	0.40					SC-K7	0.63					-	
SC-H8	0.18					SC-K8	1.09					-	
SC-H9	0.71					SC-K9	0.57					-	
SC-H10	0.44					SC-K10	0.94					-	
						SC-K11	0.29					1	
						SC-K12	0.15					1	
						SC-K13	0.41					-	
						SC-K14	0.25					-	
			1	1	1	1	1	1	1		1		







0 GF	300' RAPHIC SCALE
L	EGEND
_ · _ · _ · _	APPROXIMATE PHASE I PERMITTED WASTE BOUNDARY
	APPROXIMATE PHASE II PERMITTED WASTE BOUNDARY
10	EXISTING CONTOUR OUTSIDE LANDFILL FOOTPRINT
-90	EXISTING/PROPOSED CONTOUR WITHIN LANDFILL FOOTPRINT
	OUTFALL LOCATION
	SUBCATCHMENT AREA BOUNDARY

NOTES

1. EXISTING TOPOGRAPHY OF THE SURFACE WATER SEDIMENT CONTROL BASIN IS FROM FILE "09-034 topo.dwg," PREPARED BY SOULE AND ASSOCIATES, P.C., DATED MARCH 2009. ALL OTHER EXISTING TOPOGRAPHY PROVIDED BY WENCK ASSOCIATES, INC., 1800 PIONEER CREEK CENTER, MAPLE PLAIN MN. 55359. DATE OF AERIAL PHOTOGRAPHY: JANUARY 9, 2003.

	FICURE
FIGURE A.2-1 SUBCATCHMENT DELINEATION	1 OF 4

Appendix A.3

Weighted Curve Number Determination

SCS ENGINEERS

363	ENGINEEKJ	SHEET NO.		1 of 4	1
Job No.	25221174	CALC. NO.		3	
Job:	Indian River Landfill – Run-on/Run-off Evaluation	REV. NO.		1	
Client	NRG	ВҮ	SJL	DATE	7/10/2021
Subject	Weighted Curve Number	CHK'D.	RDS	DATE	7/30/2021

Problem Statement

Determine the weighted curve number (CN) for each subcatchment area to be modeled. The CN is used to calculate stormwater runoff for catchment areas.

<u>Given</u>

The software utilized to model the proposed closure plan conditions of the Landfill is HydroCAD. This program calculates a "weighted" curve number value for each subcatchment based on the percentage of total acreage for each soil type and land cover parameter. This method was utilized in this evaluation and is further described in the "Calculation" section below.

Please find the following supporting information attached to this calculation:

- Technical Release 55 (TR-55), Urban Hydrology for Small Watersheds, United States Department of Agriculture Natural Resources Conservation Service (USDA-NRCS).
- USDA-NRCS Custom Soil Resource Report for Sussex County, Delaware
- Surficial soil types for the all subcatchment areas associated with the Landfill were delineated using the Custom Soil Resource Report for Sussex County, Delaware published by the USDA-NRCS. As documented in this report, all subcatchments within the Landfill consist of soils from Hydrologic Soil Group A (HSG-A), consistent with native soils adjacent to the Landfill within the property. These areas are shown on **Figure A.3-1**.
- Land cover types were delineated based on the proposed closure plan conditions of the Landfill and review of aerial photography. These areas include grass cover, paved roads with open ditches, gravel roads, and water. These areas are shown on **Figure A.3-1**.
- Once delineated, the acreage for each surficial soil type and land cover type combination within each subcatchment is calculated using AutoCAD Civil 3D 2020 (AutoCAD).

Assumptions

Overview of Curve Numbers

Weighted curve numbers are used to identify the runoff characteristics of a subcatchment area. The curve number is determined by both the land cover that will be encountered by surface water (such as grass, road, etc.) as well as the type of soil that lies under the land cover. The underlying soil is important because soil matrix has a large impact on whether water infiltrates the soil or is shed.

HydroCAD utilizes curve number table values that are published by the USDA-NRCS in technical resource TR-55. The tables provide typical curve numbers for each land cover and soil group pairing.

TR-55 describes the various Hydrologic Soil Groups (HSG) as follows:

- Group A: Soils with low runoff potential; typically more than 90 percent sand or gravel.
- Group B: Moderately low runoff potential with water transmission through the soil unimpeded. Group B soils typically have between 10 and 20 percent clay and 50 to 90 percent sand and have loamy sand or sandy loam textures.

SCS	ENGINEERS
	05001174

Job No.	25221174	CALC. NO.		3	
Job:	Indian River Landfill – Run-on/Run-off Evaluation	REV. NO.		1	
Client	NRG	BY	SJL	DATE	7/10/2021
Subject	Weighted Curve Number	CHK'D.	RDS	DATE	7/30/2021

SHEET NO.

2 of 4

- Group C: Moderately high runoff potential. Typically have between 20 and 40 percent clay and less than 50 percent sand, and have loam, silt loam, sandy clay loam, clay loam, and silty clay loam textures.
- Group D: High runoff potential. Typically have greater than 40 percent clay, less than 50 percent sand, and have clayey textures.

According to the NRCS Soil Survey, all undisturbed areas are comprised of soils with Hydrologic Soil Group A, which are a group of soils that have the highest infiltration potential. Furthermore, the surficial soils that were present prior to the development of the currently permitted landfill were also Hydrologic Soil Group A soils. These soils had been stripped for use as a construction cover soil. The specific soils in the development area as identified in the NRCS Soil Survey are identified in the table below.

Sussex County, Delaware (DE005)						
Map Unit Symbol	Map Unit Name	Hydrologic Soil Group (HSG)	Percent %			
FhA	Fort Mott-Henlopen complex, 0 to 2 percent slopes	А	31.8			
FhB	Fort Mott-Henlopen complex, 2 to 5 percent slopes	А	63.5			
UzC	Udorthents, 0 to 10 percent slopes	А	4.7			

For calculation purposes, HSG A was selected as all present soils fall within this category. Specific information about each soil type is described below.

Fort Mott-Henlopen complex

The Fort Mott component makes up 45 percent of the map unit with Henlopen making up 35 percent of the map unit. The soil is excessively well drained with a depth to water table that is more than 80 inches. The soil does not flood or pond and is classified as HSG A.

Udorthents

The Udorthents component makes up nearly 90 percent of the map unit. The soil is well drained with a depth to water table that is about 40 to 72 inches. The soil does not flood or pond and is classified as HSG A.

Calculation Method

Each subcatchment area identified in **Appendix A.2** was evaluated to provide an appropriate curve number that is weighted to reflect surficial soils, land cover and antecedent moisture condition. AutoCAD was used to delineate land covers, as further described in the following text. The areas were then manually imported into HydroCAD.

SCS ENGINEERS		SHEET NO.		3 of 4	1
Job No.	25221174	CALC. NO.		3	
Job:	Indian River Landfill – Run-on/Run-off Evaluation	REV. NO.		1	
Client	NRG	BY	SJL	DATE	7/10/2021
Subject	Weighted Curve Number	CHK'D.	RDS	DATE	7/30/2021

HydroCAD then overlies the information and calculates a composite (weighted) curve number for each subcatchment area using the following equation:

$$CN_{c} = \frac{CN_{1}A_{1} + CN_{2}A_{2} \dots CN_{n}A_{n}}{A_{1} + A_{2} \dots A_{n}}$$

Where:

 $\begin{array}{ll} CN_c & = Composite \ CN \ value \\ CN_1 - CN_n & = Individual \ CN \ values \end{array}$ $A_1 - A_n$ = Area associated with each CN value

Land Cover

For proposed conditions, the land covers were determined by the proposed closure plan conditions of the Landfill and the existing land covers outside of the Landfill.

Delineated	Commente	Corresponding TR-55 Description and Runoff Coefficients					
Area	Comments	Description	Soil Group				
		Description	Α	В	С	D	
Open Space / Grass Cover	Open grassy spaces were identified for proposed conditions for vegetated areas within the Landfill and the Northeast and Southwest Basins.	Open space, Good Condition (grass cover > 75%)	39	61	74	80	
Gravel Roads	Present along the perimeter roadway of the Landfill.	Gravel roads with Right-of- Way	76	85	89	91	
Roads and Paved Areas	Present in areas with roads, where surface water is unlikely to infiltrate into the ground.	Streets and Roads, Paved, Open Ditches with Right-of- Way, 50% Impervious	83	89	92	93	
Water	Water surface present in the Northeast and Southwest Basin Forebays.	Water Surface, 0% Impervious	98	98	98	98	

Antecedent Moisture Condition (AMC)

The antecedent moisture condition indicates the moisture level in the ground immediately preceding a storm event. HydroCAD implements four AMC conditions.

- AMC 1 Dry
- AMC 2 Normal
- AMC 3 Wet
- AMC 4 Saturated or frozen

AMC 2 was used for this evaluation, which is typical engineering practice.

SCS	ENGINEERS	SHEET NO.		4 of 4	1
Job No.	25221174	CALC. NO.		3	
Job:	Indian River Landfill – Run-on/Run-off Evaluation	REV. NO.		1	
Client	NRG	BY	SJL	DATE	7/10/2021
Subject	Weighted Curve Number	CHK'D.	RDS	DATE	7/30/2021

Results

Based on the parameters and methods discussed previously, weighted curve numbers were calculated for all subcatchment areas. A summary of the weighted curve numbers for each subcatchment has been provided in **Table A.3-1**.

Table A.3-1 Determination of Weighted Curve Number								
Subcatchment Area	Landcover Type	Soil Group	Curve Number	Acreage	Percentage of Subacatchment	Weighted Curve Number		
SC-A1	>75% Grass cover, Good	A	39	0.74	100.0%	39		
SC-A2	>75% Grass cover, Good	A	39	0.39	100.0%	39		
SC-A3	>75% Grass cover, Good	A	39	0.81	100.0%	39		
SC-A4	>75% Grass cover, Good	A	39	0.80	100.0%	39		
SC-A5	>75% Grass cover, Good	A	39	0.43	100.0%	39		
50.46	>75% Grass cover, Good	A	39	0.07	15.2%	70		
30-40	Gravel roads	A	76	0.39	84.8%	- 70		
SC-A7	>75% Grass cover, Good	A	39	0.27	100.0%	39		
SC-A8	>75% Grass cover, Good	A	39	0.50	100.0%	39		
SC-A9	>75% Grass cover, Good	A	39	0.68	100.0%	39		
SC 110	>75% Grass cover, Good	A	39	0.31	84.4%	45		
30410	Gravel roads	A	76	0.06	15.6%	42		
SC-A11	>75% Grass cover, Good	A	39	0.72	100.0%	39		
SC 112	>75% Grass cover, Good	A	39	0.12	68.2%	51		
30-A12	Gravel roads	A	76	0.05	31.8%	51		
SC-B1	>75% Grass cover, Good	A	39	0.43	100.0%	39		
SC-B2	>75% Grass cover, Good	A	39	0.89	100.0%	39		
SC-B3	>75% Grass cover, Good	A	39	3.16	100.0%	39		
SC-B4	>75% Grass cover, Good	A	39	1.18	100.0%	39		
SC-B5	>75% Grass cover, Good	A	39	1.27	100.0%	39		
SC-B6	>75% Grass cover, Good	A	39	0.03	100.0%	39		
SC-B7	>75% Grass cover, Good	A	39	0.46	100.0%	39		
CC 89	>75% Grass cover, Good	A	39	0.08	15.5%	70		
30-80	Gravel roads	A	76	0.45	84.5%	10		
SC-B9	>75% Grass cover, Good	A	39	0.13	100.0%	39		
SC-810	>75% Grass cover, Good	A	39	0.46	96.7%	40		
00 510	Gravel roads	A	76	0.02	3.3%	40		
SC-B11	>75% Grass cover, Good	A	39	0.01	62.8%	53		
00 511	Gravel roads	A	76	0.01	37.2%			
SC-B12	>75% Grass cover, Good	A	39	1.00	100.0%	39		
SC-B13	>75% Grass cover, Good	A	39	0.28	100.0%	39		
SC-B14	>75% Grass cover, Good	A	39	1.11	100.0%	39		
SC-B15	>75% Grass cover, Good	A	39	0.59	100.0%	39		
SC-B16	>75% Grass cover, Good	A	39	0.33	100.0%	39		
SC-P17	>75% Grass cover, Good	A	39	0.11	66.8%	51		
SC-B17	Gravel roads	А	76	0.05	33.2%	51		



Table A.3-1 Determination of Weighted Curve Number								
Subcatchment Area	Landcover Type	Soll Group	Curve Number	Acreage	Percentage of Subacatchment	Weighted Curve Number		
SC-C1	>75% Grass cover, Good	A	39	0.42	100.0%	39		
SC-C2	>75% Grass cover, Good	A	39	0.04	100.0%	39		
SC-C3	>75% Grass cover, Good	A	39	0.55	100.0%	39		
SC-C4	>75% Grass cover, Good	A	39	0.31	100.0%	39		
SC-C5	>75% Grass cover, Good	A	39	0.52	100.0%	39		
SC-C6	>75% Grass cover, Good	A	39	0.31	100.0%	39		
SC-C7	>75% Grass cover, Good	A	39	0.48	100.0%	39		
SC-C8	>75% Grass cover, Good	A	39	0.30	100.0%	39		
SC-C9	>75% Grass cover, Good	A	39	0.06	100.0%	39		
	>75% Grass cover, Good	A	39	0.04	67.7%			
SC-C10	Gravel roads	A	76	0.02	32.3%	51		
SC-D1	>75% Grass cover, Good	A	39	1.78	100.0%	39		
SC-D2	>75% Grass cover, Good	A	39	0.10	100.0%	39		
SC-D3	>75% Grass cover, Good	A	39	1.13	100.0%	39		
SC-D4	>75% Grass cover, Good	A	39	1.01	100.0%	39		
SC-D5	>75% Grass cover, Good	A	39	1.05	100.0%	39		
SC-D6	>75% Grass cover, Good	A	39	0.14	100.0%	39		
	>75% Grass cover, Good	A	39	0.05	69.1%			
SC-D7	Gravel roads	A	76	0.02	30.9%	50		
SC-E1	>75% Grass cover, Good	A	39	5.74	100.0%	39		
SC-E2	>75% Grass cover, Good	A	39	0.15	100.0%	39		
SC-E3	>75% Grass cover, Good	A	39	0.31	100.0%	39		
SC-E4	>75% Grass cover, Good	A	39	0.39	100.0%	39		
SC-E5	>75% Grass cover, Good	A	39	0.46	100.0%	39		
50 F6	>75% Grass cover, Good	A	39	0.09	34.2%	63		
5C-E6	Gravel roads	A	76	0.18	65.8%	63		
SC-E7	>75% Grass cover, Good	A	39	0.22	100.0%	39		
SC-F8	>75% Grass cover, Good	A	39	0.06	49.1%	58		
	Gravel roads	A	76	0.06	50.9%			
SC-F1	>75% Grass cover, Good	A	39	0.97	100.0%	39		
SC-F2	>75% Grass cover, Good	A	39	1.22	100.0%	39		
SC-F3	>75% Grass cover, Good	A	39	1.36	100.0%	39		
SC-F4	>75% Grass cover, Good	A	39	0.85	100.0%	39		
SC-F5	>75% Grass cover, Good	A	39	0.16	36.0%	63		
	Gravel roads	А	76	0.28	64.0%			



Table A.3-1 Determination of Weighted Curve Number								
Subcatchment Area	Landcover Type	Soli Group	Curve Number	Acreage	Percentage of Subacatchment	Weighted Curve Number		
SC-G1	>75% Grass cover, Good	A	39	0.59	100.0%	39		
SC-G2	>75% Grass cover, Good	A	39	1.10	100.0%	39		
SC-G3	>75% Grass cover, Good	A	39	0.43	100.0%	39		
SC-G4	>75% Grass cover, Good	A	39	0.40	100.0%	39		
SC-G5	>75% Grass cover, Good	A	39	0.44	100.0%	39		
SC-G6	>75% Grass cover, Good	A	39	0.40	100.0%	39		
SC-G7	>75% Grass cover, Good	A	39	0.22	100.0%	39		
50.08	>75% Grass cover, Good	A	39	0.04	35.9%	63		
50-68	Gravel roads	A	76	0.07	64.1%	63		
SC-G9	>75% Grass cover, Good	A	39	0.81	100.0%	39		
60.010	>75% Grass cover, Good	A	39	0.10	36.8%	60		
50-610	Gravel roads	A	76	0.17	63.2%	62		
SC-H1	>75% Grass cover, Good	A	39	0.54	100.0%	39		
SC-H2	>75% Grass cover, Good	A	39	0.78	100.0%	39		
SC-H3	>75% Grass cover, Good	A	39	0.82	100.0%	39		
SC-H4	>75% Grass cover, Good	A	39	0.71	100.0%	39		
SC-H5	>75% Grass cover, Good	A	39	0.98	100.0%	39		
SC-H6	>75% Grass cover, Good	A	39	0.93	100.0%	39		
SC-H7	>75% Grass cover, Good	A	39	0.40	100.0%	39		
	>75% Grass cover, Good	A	39	0.06	31.4%			
SC-H8	Gravel roads	A	76	0.08	46.9%	66		
	Paved Roads	A	83	0.04	21.7%			
SC-H9	>75% Grass cover, Good	A	39	0.71	100.0%	39		
SC 110	>75% Grass cover, Good	A	39	0.15	34.9%	68		
30-110	Paved Roads	A	83	0.29	65.1%	00		
SC-I1	>75% Grass cover, Good	A	39	1.91	100.0%	39		
SC-12	>75% Grass cover, Good	A	39	0.54	100.0%	39		
SC-I3	>75% Grass cover, Good	A	39	0.18	100.0%	39		
SC-14	>75% Grass cover, Good	A	39	0.06	100.0%	39		
SC-15	>75% Grass cover, Good	A	39	0.40	100.0%	39		
50.16	>75% Grass cover, Good	A	39	0.07	45.5%	63		
30-10	Paved Roads	A	83	0.09	54.5%	65		
SC-J1	>75% Grass cover, Good	A	39	2.81	100.0%	39		
SC-J2	>75% Grass cover, Good	A	39	0.14	100.0%	39		
SC-J3	>75% Grass cover, Good	A	39	0.29	100.0%	39		
SC-J4	>75% Grass cover, Good	A	39	0.40	100.0%	39		
SC-J5	>75% Grass cover, Good	A	39	0.42	100.0%	39		
00.10	>75% Grass cover, Good	A	39	0.10	40.2%	65		
26-70	Paved Roads	A	83	0.15	59.8%	69		



Table A.3-1 Determination of Weighted Curve Number							
Subcatchment Area	Landcover Type	Soil Group	Curve Number	Acreage	Percentage of Subacatchment	Weighted Curve Number	
SC-K1	>75% Grass cover, Good	А	39	2.80	100.0%	39	
SC-K2	>75% Grass cover, Good	A	39	1.40	100.0%	39	
SC-K3	>75% Grass cover, Good	A	39	0.37	100.0%	39	
SC-K4	>75% Grass cover, Good	A	39	0.66	100.0%	39	
SC-K5	>75% Grass cover, Good	A	39	0.69	100.0%	39	
SC-K6	>75% Grass cover, Good	A	39	1.11	100.0%	39	
SC-K7	>75% Grass cover, Good	A	39	0.63	100.0%	39	
SC-K8	>75% Grass cover, Good	A	39	1.09	100.0%	39	
SC-K9	>75% Grass cover, Good	A	39	0.57	100.0%	39	
SC-K10	>75% Grass cover, Good	A	39	0.94	100.0%	39	
SC-K11	>75% Grass cover, Good	A	39	0.29	100.0%	39	
	>75% Grass cover, Good	A	39	0.06	40.7%	64	
SC-K12	Gravel roads	A	76	0.04	23.6%		
	Paved Roads	A	83	0.05	35.7%		
SC-K13	>75% Grass cover, Good	A	39	0.41	100.0%	39	
SC 1/14	>75% Grass cover, Good	A	39	0.05	20.2%		
30-R14	Gravel roads	A	76	0.20	79.8%	69	
SC NE1	>75% Grass cover, Good	A	39	0.30	92.5%	42	
JUNEI	Gravel roads	A	76	0.02	7.5%	42	
SC-NE2	Water Surface, 0% imp	A	98	0.50	100.0%	98	
	>75% Grass cover, Good	A	39	0.36	75.8%		
SC-NE3	Gravel roads	A	76	0.04	9.3%	49	
	Paved Roads	A	83	0.07	14.8%		
SC-NE4	Water Surface, 0% imp	A	98	0.46	100.0%	98	
00.1/55	>75% Grass cover, Good	А	39	7.76	96.5%	10	
SC-NE5	Gravel roads	A	76	0.28	3.5%	40	
SC-SW1	>75% Grass cover, Good	A	39	0.17	100.0%	39	
SC-SW2	Water Surface, 0% imp	A	98	0.40	100.0%	98	
SC-SW3	>75% Grass cover, Good	A	39	0.45	100.0%	39	
SC-SW4	>75% Grass cover, Good	A	39	3.20	100.0%	39	





0 G	300' GRAPHIC SCALE
Ĺ	EGEND
· _ · _ · _ · _ · _	APPROXIMATE PHASE I PERMITTED WASTE BOUNDARY
	APPROXIMATE PHASE II PERMITTED WASTE BOUNDARY
	EXISTING CONTOUR OUTSIDE LANDFILL FOOTPRINT
90	EXISTING/PROPOSED CONTOUR WITHIN LANDFILL FOOTPRINT
	OUTFALL LOCATION
	SUBCATCHMENT AREA BOUNDARY
	SURFICIAL SOIL TYPE BOUNDARY (USDA-NRCS CUSTOM SOIL REPORT)
	OPEN SPACE / GRASS COVER
	GRAVEL ROADS
	PAVED ROADS
	WATER

NOTES

1. EXISTING TOPOGRAPHY OF THE SURFACE WATER SEDIMENT CONTROL BASIN IS FROM FILE "09-034 topo.dwg," PREPARED BY SOULE AND ASSOCIATES, P.C., DATED MARCH 2009. ALL OTHER EXISTING TOPOGRAPHY PROVIDED BY WENCK ASSOCIATES, INC., 1800 PIONEER CREEK CENTER, MAPLE PLAIN MN. 55359. DATE OF AERIAL PHOTOGRAPHY: JANUARY 9, 2003.

FIGURE A.3–1 SURFICIAL SOIL TYPES AND LAND COVER	FIGURE
	2 OF 4



United States Department of Agriculture

Natural Resources Conservation

Service

A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

Custom Soil Resource Report for Sussex County, Delaware



Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (https://offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/? cid=nrcs142p2_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

The U.S. Department of Agriculture (USDA) prohibits discrimination in all its programs and activities on the basis of race, color, national origin, age, disability, and where applicable, sex, marital status, familial status, parental status, religion, sexual orientation, genetic information, political beliefs, reprisal, or because all or a part of an individual's income is derived from any public assistance program. (Not all prohibited bases apply to all programs.) Persons with disabilities who require

alternative means for communication of program information (Braille, large print, audiotape, etc.) should contact USDA's TARGET Center at (202) 720-2600 (voice and TDD). To file a complaint of discrimination, write to USDA, Director, Office of Civil Rights, 1400 Independence Avenue, S.W., Washington, D.C. 20250-9410 or call (800) 795-3272 (voice) or (202) 720-6382 (TDD). USDA is an equal opportunity provider and employer.

Contents

Preface	2
How Soil Surveys Are Made	5
Soil Map	8
Soil Map	9
Legend	10
Map Unit Legend	11
Map Unit Descriptions	11
Sussex County, Delaware	14
BuA—Brockatonorton-Urban land complex, 0 to 2 percent slopes	14
EvD—Evesboro loamy sand, 5 to 15 percent slopes	15
FhA—Fort Mott-Henlopen complex, 0 to 2 percent slopes	17
FhB—Fort Mott-Henlopen complex, 2 to 5 percent slopes	19
HpA—Henlopen loamy sand, 0 to 2 percent slopes	21
HpB—Henlopen loamy sand, 2 to 5 percent slopes	22
KsA—Klej loamy sand, 0 to 2 percent slopes	23
Ma—Manahawkin muck, frequently flooded	25
Pa—Pawcatuck mucky peat, very frequently flooded, tidal	26
RoA—Rosedale loamy sand, 0 to 2 percent slopes	28
RuA—Runclint loamy sand, 0 to 2 percent slopes	29
UzC—Udorthents, 0 to 10 percent slopes	31
WHe1—Herring Creek mucky silt loam, 0 to 1 meter water depth	32
References	34
How Soil Surveys Are Made

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.



	MAP L	EGEND		MAP INFORMATION			
Area of Int	Area of Interest (AOI)		Spoil Area	The soil surveys that comprise your AOI were mapped at			
	Area of Interest (AOI)	۵	Stony Spot	1:24,000.			
Soils		0	Very Stony Spot	Warning: Soil Man may not be valid at this scale			
	Soil Map Unit Polygons	Ŷ	Wet Spot	Warning. Ool wap may not be valid at this seale.			
~	Soil Map Unit Lines	8 A	Other	Enlargement of maps beyond the scale of mapping can cause			
	Soil Map Unit Points		Special Line Features	line placement. The maps do not show the small areas of			
Special I	Special Point Features		turos	contrasting soils that could have been shown at a more detailed			
ు	Blowout		Streams and Canals	scale.			
\boxtimes	Borrow Pit	Transport	ation	Please rely on the bar scale on each man sheet for man			
×	Clay Spot		Rails	measurements.			
\diamond	Closed Depression	~	Interstate Highways	Source of Many Natural Descurses Concernation Service			
X	Gravel Pit	~	US Routes	Web Soil Survey URL:			
00	Gravelly Spot	~	Major Roads	Coordinate System: Web Mercator (EPSG:3857)			
٥	Landfill	-	Local Roads	Maps from the Web Soil Survey are based on the Web Mercator			
٨.	Lava Flow	Backgrou	nd	projection, which preserves direction and shape but distorts			
عليه	Marsh or swamp	in the second se	Aerial Photography	distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more			
\$	Mine or Quarry			accurate calculations of distance or area are required.			
0	Miscellaneous Water			This product is generated from the USDA-NRCS certified data as			
õ	Perennial Water			of the version date(s) listed below.			
Ň	Rock Outcrop			Soil Survey Area: Sursay County Delaware			
Ť	Saline Spot			Survey Area Data: Version 21, Jun 11, 2020			
•.•	Sandy Spot			Sail man unite are labeled (as apage allows) for man apples			
-	Severely Eroded Spot			1:50,000 or larger.			
	Sinkhole			Data(a) assist images were photographed. Nev 21, 2019. Mar			
ě.	Slide or Slip			12, 2019			
₽° ¢	Sodic Spot						
<i>کر</i>	·			one orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.			

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI			
BuA	Brockatonorton-Urban land complex, 0 to 2 percent slopes	79.5	11.2%			
EvD	Evesboro loamy sand, 5 to 15 percent slopes	1.9	0.3%			
FhA	Fort Mott-Henlopen complex, 0 to 2 percent slopes	210.0	29.6%			
FhB	Fort Mott-Henlopen complex, 2 to 5 percent slopes	140.1	19.8%			
НрА	Henlopen loamy sand, 0 to 2 percent slopes	36.5	5.1%			
НрВ	Henlopen loamy sand, 2 to 5 percent slopes	37.1	5.2%			
KsA	Klej loamy sand, 0 to 2 percent slopes	8.7	1.2%			
Ма	Manahawkin muck, frequently flooded	27.9	3.9%			
Ра	Pawcatuck mucky peat, very frequently flooded, tidal	26.1	3.7%			
RoA	Rosedale loamy sand, 0 to 2 percent slopes	9.7	1.4%			
RuA	Runclint loamy sand, 0 to 2 percent slopes	15.9	2.2%			
UzC	Udorthents, 0 to 10 percent slopes	89.0	12.6%			
WHe1	Herring Creek mucky silt loam, 0 to 1 meter water depth	26.1	3.7%			
Totals for Area of Interest		708.6	100.0%			

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example. An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

Sussex County, Delaware

BuA—Brockatonorton-Urban land complex, 0 to 2 percent slopes

Map Unit Setting

National map unit symbol: 1qtfp Elevation: 0 to 130 feet Mean annual precipitation: 42 to 48 inches Mean annual air temperature: 52 to 58 degrees F Frost-free period: 180 to 220 days Farmland classification: Not prime farmland

Map Unit Composition

Brockatonorton and similar soils: 45 percent Urban land: 35 percent Minor components: 20 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Brockatonorton

Setting

Landform: Back-barrier beaches Down-slope shape: Concave Across-slope shape: Linear Parent material: Sandy eolian deposits and/or sandy marine deposits

Typical profile

A - 0 to 3 inches: sand C - 3 to 24 inches: sand Cg1 - 24 to 50 inches: sand Oe - 50 to 60 inches: mucky peat Cg2 - 60 to 72 inches: sand

Properties and qualities

Slope: 0 to 2 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Moderately well drained
Runoff class: Very low
Capacity of the most limiting layer to transmit water (Ksat): High to very high (1.98 to 19.98 in/hr)
Depth to water table: About 24 to 36 inches
Frequency of flooding: Occasional
Frequency of ponding: None
Maximum salinity: Nonsaline to strongly saline (0.0 to 16.0 mmhos/cm)
Available water capacity: Low (about 5.7 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 6s Hydrologic Soil Group: A Hydric soil rating: No

Description of Urban Land

Setting

Landform: Flats

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 8 Hydric soil rating: Unranked

Minor Components

Udorthents

Percent of map unit: 10 percent Landform: Flats Hydric soil rating: No

Beaches

Percent of map unit: 5 percent Landform: Beaches Hydric soil rating: Yes

Acquango

Percent of map unit: 3 percent Landform: Backshores, dunes Hydric soil rating: No

Transquaking

Percent of map unit: 2 percent Landform: Tidal marshes Hydric soil rating: Yes

EvD—Evesboro loamy sand, 5 to 15 percent slopes

Map Unit Setting

National map unit symbol: 1qtgc Elevation: 0 to 200 feet Mean annual precipitation: 42 to 48 inches Mean annual air temperature: 52 to 58 degrees F Frost-free period: 180 to 220 days Farmland classification: Not prime farmland

Map Unit Composition

Evesboro and similar soils: 75 percent *Minor components:* 25 percent *Estimates are based on observations, descriptions, and transects of the mapunit.*

Description of Evesboro

Setting

Landform: Flats, knolls, fluviomarine terraces, dunes Down-slope shape: Linear, convex Across-slope shape: Linear, convex Parent material: Sandy eolian deposits and/or fluviomarine sediments

Typical profile

Ap - 0 to 4 inches: loamy sand E - 4 to 16 inches: loamy sand Bw - 16 to 39 inches: loamy sand C - 39 to 80 inches: sand

Properties and qualities

Slope: 5 to 15 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Excessively drained
Runoff class: Negligible
Capacity of the most limiting layer to transmit water (Ksat): High to very high (5.95 to 99.90 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Low (about 3.9 inches)

Interpretive groups

Land capability classification (irrigated): 6e Land capability classification (nonirrigated): 6e Hydrologic Soil Group: A Hydric soil rating: No

Minor Components

Runclint

Percent of map unit: 10 percent Landform: Flats, fluviomarine terraces, dunes, knolls Landform position (three-dimensional): Rise Down-slope shape: Linear, convex Across-slope shape: Linear, convex Hydric soil rating: No

Galloway

Percent of map unit: 5 percent Landform: Depressions, flats Down-slope shape: Concave, linear Across-slope shape: Concave, linear Hydric soil rating: No

Cedartown

Percent of map unit: 5 percent Landform: Knolls, dunes, flats Landform position (three-dimensional): Rise, talf Down-slope shape: Convex, linear Across-slope shape: Convex, linear Hydric soil rating: No

Fort mott

Percent of map unit: 5 percent Landform: Fluviomarine terraces, flats, knolls Landform position (three-dimensional): Rise Down-slope shape: Linear, convex Across-slope shape: Linear, convex Hydric soil rating: No

FhA—Fort Mott-Henlopen complex, 0 to 2 percent slopes

Map Unit Setting

National map unit symbol: 1qtgh Elevation: 20 to 70 feet Mean annual precipitation: 42 to 48 inches Mean annual air temperature: 52 to 58 degrees F Frost-free period: 180 to 220 days Farmland classification: Prime farmland if irrigated

Map Unit Composition

Fort mott and similar soils: 45 percent Henlopen and similar soils: 35 percent Minor components: 20 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Fort Mott

Setting

Landform: Flats, fluviomarine terraces Down-slope shape: Linear Across-slope shape: Linear Parent material: Sandy eolian deposits over fluviomarine sediments

Typical profile

Ap - 0 to 10 inches: loamy sand E - 10 to 24 inches: loamy sand Bt - 24 to 36 inches: sandy loam C - 36 to 80 inches: loamy sand

Properties and qualities

Slope: 0 to 2 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Runoff class: Negligible
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (1.28 to 5.95 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Low (about 5.3 inches)

Interpretive groups

Land capability classification (irrigated): 2s Land capability classification (nonirrigated): 2s Hydrologic Soil Group: A Hydric soil rating: No

Description of Henlopen

Setting

Landform: Marine terraces, dunes Down-slope shape: Convex Across-slope shape: Linear Parent material: Sandy eolian deposits and loamy fluviomarine sediments

Typical profile

Ap - 0 to 10 inches: loamy sand E - 10 to 46 inches: loamy sand Bt - 46 to 62 inches: sandy loam C - 62 to 80 inches: sand

Properties and qualities

Slope: 0 to 2 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Somewhat excessively drained
Runoff class: Negligible
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.57 to 5.95 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Low (about 5.9 inches)

Interpretive groups

Land capability classification (irrigated): 2s Land capability classification (nonirrigated): 3s Hydrologic Soil Group: A Hydric soil rating: No

Minor Components

Rosedale

Percent of map unit: 5 percent Landform: Flats, knolls Hydric soil rating: No

Downer

Percent of map unit: 5 percent Landform: Flats Hydric soil rating: No

Ingleside

Percent of map unit: 5 percent Landform: Flats Hydric soil rating: No

Runclint

Percent of map unit: 5 percent Landform: Dunes, knolls, flats Hydric soil rating: No

FhB—Fort Mott-Henlopen complex, 2 to 5 percent slopes

Map Unit Setting

National map unit symbol: 1qtgj Elevation: 20 to 70 feet Mean annual precipitation: 42 to 48 inches Mean annual air temperature: 52 to 58 degrees F Frost-free period: 180 to 220 days Farmland classification: Prime farmland if irrigated

Map Unit Composition

Fort mott and similar soils: 45 percent Henlopen and similar soils: 35 percent Minor components: 20 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Fort Mott

Setting

Landform: Flats, knolls, fluviomarine terraces Landform position (three-dimensional): Rise Down-slope shape: Linear, convex Across-slope shape: Linear, convex Parent material: Sandy eolian deposits over fluviomarine sediments

Typical profile

Ap - 0 to 10 inches: loamy sand E - 10 to 24 inches: loamy sand Bt - 24 to 36 inches: sandy loam C - 36 to 80 inches: loamy sand

Properties and qualities

Slope: 2 to 5 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Runoff class: Negligible
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (1.28 to 5.95 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Low (about 5.3 inches)

Interpretive groups

Land capability classification (irrigated): 2s Land capability classification (nonirrigated): 2s Hydrologic Soil Group: A Hydric soil rating: No

Description of Henlopen

Setting

Landform: Marine terraces, dunes Down-slope shape: Linear, convex Across-slope shape: Linear Parent material: Sandy eolian deposits and loamy fluviomarine sediments

Typical profile

Ap - 0 to 10 inches: loamy sand E - 10 to 46 inches: loamy sand Bt - 46 to 62 inches: sandy loam C - 62 to 80 inches: sand

Properties and qualities

Slope: 2 to 5 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Somewhat excessively drained
Runoff class: Negligible
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.57 to 5.95 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Low (about 5.9 inches)

Interpretive groups

Land capability classification (irrigated): 2s Land capability classification (nonirrigated): 3s Hydrologic Soil Group: A Hydric soil rating: No

Minor Components

Rosedale

Percent of map unit: 5 percent Landform: Flats, knolls Hydric soil rating: No

Runclint

Percent of map unit: 5 percent Landform: Knolls, flats, dunes Hydric soil rating: No

Downer

Percent of map unit: 5 percent Landform: Flats Hydric soil rating: No

Ingleside

Percent of map unit: 5 percent Landform: Flats Hydric soil rating: No

HpA—Henlopen loamy sand, 0 to 2 percent slopes

Map Unit Setting

National map unit symbol: 1qth3 Elevation: 20 to 70 feet Mean annual precipitation: 42 to 48 inches Mean annual air temperature: 52 to 58 degrees F Frost-free period: 180 to 220 days Farmland classification: Prime farmland if irrigated

Map Unit Composition

Henlopen and similar soils: 80 percent *Minor components:* 20 percent *Estimates are based on observations, descriptions, and transects of the mapunit.*

Description of Henlopen

Setting

Landform: Marine terraces, dunes Down-slope shape: Linear, convex Across-slope shape: Linear Parent material: Sandy eolian deposits and loamy fluviomarine sediments

Typical profile

Ap - 0 to 10 inches: loamy sand E - 10 to 46 inches: loamy sand Bt - 46 to 62 inches: sandy loam C - 62 to 80 inches: sand

Properties and qualities

Slope: 0 to 2 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Somewhat excessively drained
Runoff class: Negligible
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.57 to 5.95 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Low (about 5.9 inches)

Interpretive groups

Land capability classification (irrigated): 2s Land capability classification (nonirrigated): 3s Hydrologic Soil Group: A Hydric soil rating: No

Minor Components

Rosedale

Percent of map unit: 5 percent Landform: Flats, knolls Hydric soil rating: No

Runclint

Percent of map unit: 5 percent Landform: Knolls, flats, dunes Hydric soil rating: No

Ingleside

Percent of map unit: 5 percent Landform: Flats Hydric soil rating: No

Fort mott

Percent of map unit: 5 percent Landform: Flats Hydric soil rating: No

HpB—Henlopen loamy sand, 2 to 5 percent slopes

Map Unit Setting

National map unit symbol: 1qth4 Elevation: 20 to 70 feet Mean annual precipitation: 42 to 48 inches Mean annual air temperature: 52 to 58 degrees F Frost-free period: 180 to 220 days Farmland classification: Prime farmland if irrigated

Map Unit Composition

Henlopen and similar soils: 80 percent *Minor components:* 20 percent *Estimates are based on observations, descriptions, and transects of the mapunit.*

Description of Henlopen

Setting

Landform: Marine terraces, dunes Down-slope shape: Linear, convex Across-slope shape: Linear Parent material: Sandy eolian deposits and loamy fluviomarine sediments

Typical profile

Ap - 0 to 10 inches: loamy sand E - 10 to 46 inches: loamy sand Bt - 46 to 62 inches: sandy loam C - 62 to 80 inches: sand

Properties and qualities

Slope: 2 to 5 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Somewhat excessively drained
Runoff class: Negligible
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.57 to 5.95 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Low (about 5.9 inches)

Interpretive groups

Land capability classification (irrigated): 2s Land capability classification (nonirrigated): 3s Hydrologic Soil Group: A Hydric soil rating: No

Minor Components

Rosedale

Percent of map unit: 5 percent Landform: Flats, knolls Hydric soil rating: No

Ingleside

Percent of map unit: 5 percent Landform: Flats Hydric soil rating: No

Fort mott

Percent of map unit: 5 percent Landform: Flats Hydric soil rating: No

Runclint

Percent of map unit: 5 percent *Landform:* Dunes, knolls, flats *Hydric soil rating:* No

KsA—Klej loamy sand, 0 to 2 percent slopes

Map Unit Setting

National map unit symbol: 1qthw Elevation: 0 to 200 feet Mean annual precipitation: 42 to 48 inches Mean annual air temperature: 52 to 58 degrees F Frost-free period: 180 to 220 days Farmland classification: Farmland of statewide importance

Map Unit Composition

Klej and similar soils: 70 percent *Minor components:* 30 percent *Estimates are based on observations, descriptions, and transects of the mapunit.*

Description of Klej

Setting

Landform: Flats, depressions Down-slope shape: Linear, concave Across-slope shape: Linear, concave Parent material: Sandy eolian deposits and/or fluviomarine sediments

Typical profile

A - 0 to 7 inches: loamy sand E - 7 to 14 inches: loamy sand Bw - 14 to 20 inches: loamy sand C - 20 to 62 inches: loamy sand Cg - 62 to 80 inches: sand

Properties and qualities

Slope: 0 to 2 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Somewhat poorly drained
Runoff class: Negligible
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to very high (0.57 to 19.98 in/hr)
Depth to water table: About 10 to 20 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Low (about 4.4 inches)

Interpretive groups

Land capability classification (irrigated): 3w Land capability classification (nonirrigated): 3w Hydrologic Soil Group: A/D Hydric soil rating: No

Minor Components

Galloway

Percent of map unit: 10 percent Landform: Depressions, flats Down-slope shape: Concave, linear Across-slope shape: Concave, linear Hydric soil rating: No

Hammonton

Percent of map unit: 5 percent Landform: Flats, depressions, drainageways Down-slope shape: Linear, concave Across-slope shape: Linear, concave Hydric soil rating: No

Runclint

Percent of map unit: 5 percent *Landform:* Knolls, flats, fluviomarine terraces, dunes Landform position (three-dimensional): Rise Down-slope shape: Convex, linear Across-slope shape: Convex, linear Hydric soil rating: No

Berryland, drained

Percent of map unit: 5 percent Landform: Depressions, flats, swales Landform position (three-dimensional): Talf Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: Yes

Hurlock, drained

Percent of map unit: 5 percent Landform: Depressions, flats, swales Landform position (three-dimensional): Dip Down-slope shape: Concave, linear Across-slope shape: Concave, linear Hydric soil rating: Yes

Ma—Manahawkin muck, frequently flooded

Map Unit Setting

National map unit symbol: 1qtj3 Elevation: 0 to 100 feet Mean annual precipitation: 42 to 48 inches Mean annual air temperature: 52 to 58 degrees F Frost-free period: 180 to 220 days Farmland classification: Not prime farmland

Map Unit Composition

Manahawkin and similar soils: 85 percent Minor components: 15 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Manahawkin

Setting

Landform: Swamps, flood plains Down-slope shape: Linear Across-slope shape: Linear Parent material: Organic, woody material over sandy alluvium

Typical profile

Oa1 - 0 to 8 inches: muck Oa2 - 8 to 40 inches: muck Cg - 40 to 80 inches: sand

Properties and qualities

Slope: 0 to 1 percent

Depth to restrictive feature: More than 80 inches
Drainage class: Very poorly drained
Runoff class: Negligible
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to very high (1.28 to 19.98 in/hr)
Depth to water table: About 0 to 5 inches
Frequency of flooding: Frequent
Frequency of ponding: Frequent
Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Available water capacity: Very high (about 17.4 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 7w Hydrologic Soil Group: A/D Hydric soil rating: Yes

Minor Components

Puckum

Percent of map unit: 10 percent Landform: Flood plains, swamps, depressions Down-slope shape: Linear, concave Across-slope shape: Linear, concave Hydric soil rating: Yes

Indiantown

Percent of map unit: 5 percent Landform: Flood plains Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: Yes

Pa—Pawcatuck mucky peat, very frequently flooded, tidal

Map Unit Setting

National map unit symbol: 1qtjf Elevation: 0 feet Mean annual precipitation: 42 to 48 inches Mean annual air temperature: 52 to 58 degrees F Frost-free period: 180 to 220 days Farmland classification: Not prime farmland

Map Unit Composition

Pawcatuck, very frequently flooded, and similar soils: 80 percent Minor components: 20 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Pawcatuck, Very Frequently Flooded

Setting

Landform: Tidal flats Down-slope shape: Linear Across-slope shape: Linear Parent material: Herbaceous organic material over sandy marine deposits

Typical profile

Oe1 - 0 to 14 inches: mucky peat *Oe2 - 14 to 45 inches:* mucky peat *Cg1 - 45 to 50 inches:* loamy sand *Cg2 - 50 to 90 inches:* sand

Properties and qualities

Slope: 0 to 1 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Very poorly drained
Runoff class: Negligible
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to very high (0.57 to 19.98 in/hr)
Depth to water table: About 0 inches
Frequency of flooding: Very frequent
Frequency of ponding: Frequent
Maximum salinity: Very slightly saline to strongly saline (2.0 to 32.0 mmhos/cm)
Available water capacity: Very high (about 13.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 8 Hydrologic Soil Group: A/D Hydric soil rating: Yes

Minor Components

Transquaking

Percent of map unit: 10 percent Landform: Tidal marshes Hydric soil rating: Yes

Sunken

Percent of map unit: 5 percent *Landform:* Submerged upland tidal marshes *Hydric soil rating:* Yes

Mispillion

Percent of map unit: 5 percent *Landform:* Tidal marshes, flood plains *Hydric soil rating:* Yes

RoA—Rosedale loamy sand, 0 to 2 percent slopes

Map Unit Setting

National map unit symbol: 1qtjx Elevation: 0 to 120 feet Mean annual precipitation: 42 to 48 inches Mean annual air temperature: 52 to 58 degrees F Frost-free period: 180 to 220 days Farmland classification: Prime farmland if irrigated

Map Unit Composition

Rosedale and similar soils: 75 percent Minor components: 25 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Rosedale

Setting

Landform: Flats Down-slope shape: Linear Across-slope shape: Linear Parent material: Sandy eolian deposits over fluviomarine deposits

Typical profile

A - 0 to 9 inches: loamy sand E - 9 to 25 inches: loamy sand Bt - 25 to 38 inches: sandy loam C - 38 to 68 inches: loamy sand 2Cg - 68 to 80 inches: sandy clay loam

Properties and qualities

Slope: 0 to 2 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Runoff class: Negligible
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.20 to 5.95 in/hr)
Depth to water table: About 40 to 72 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Low (about 5.3 inches)

Interpretive groups

Land capability classification (irrigated): 2s Land capability classification (nonirrigated): 2s Hydrologic Soil Group: A Hydric soil rating: No

Minor Components

Evesboro

Percent of map unit: 10 percent Landform: Flats Landform position (three-dimensional): Talf Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: No

Galloway

Percent of map unit: 5 percent Landform: Depressions, flats Down-slope shape: Concave, linear Across-slope shape: Concave, linear Hydric soil rating: No

Klej

Percent of map unit: 5 percent Landform: Depressions, flats Down-slope shape: Concave, linear Across-slope shape: Concave, linear Hydric soil rating: No

Hambrook

Percent of map unit: 5 percent Landform: Fluviomarine terraces, flats, depressions Down-slope shape: Linear, concave Across-slope shape: Linear, concave Hydric soil rating: No

RuA—Runclint loamy sand, 0 to 2 percent slopes

Map Unit Setting

National map unit symbol: 1qtjz Elevation: 0 to 120 feet Mean annual precipitation: 42 to 48 inches Mean annual air temperature: 52 to 58 degrees F Frost-free period: 180 to 220 days Farmland classification: Not prime farmland

Map Unit Composition

Runclint and similar soils: 75 percent *Minor components:* 25 percent *Estimates are based on observations, descriptions, and transects of the mapunit.*

Description of Runclint

Setting

Landform: Flats, fluviomarine terraces

Down-slope shape: Linear *Across-slope shape:* Linear *Parent material:* Sandy eolian deposits and/or fluviomarine sediments

Typical profile

Ap - 0 to 9 inches: loamy sand E - 9 to 22 inches: sand Bw - 22 to 39 inches: sand BC - 39 to 59 inches: sand 2C - 59 to 80 inches: loamy coarse sand

Properties and qualities

Slope: 0 to 2 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Excessively drained
Runoff class: Negligible
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to very high (0.57 to 19.98 in/hr)
Depth to water table: About 40 to 72 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Low (about 3.5 inches)

Interpretive groups

Land capability classification (irrigated): 3s Land capability classification (nonirrigated): 4s Hydrologic Soil Group: A Hydric soil rating: No

Minor Components

Evesboro

Percent of map unit: 10 percent Landform: Flats Landform position (three-dimensional): Talf Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: No

Klej

Percent of map unit: 5 percent Landform: Flats, depressions Down-slope shape: Linear, concave Across-slope shape: Linear, concave Hydric soil rating: No

Hurlock, drained

Percent of map unit: 5 percent Landform: Depressions, flats, swales Landform position (three-dimensional): Dip Down-slope shape: Concave, linear Across-slope shape: Concave, linear Hydric soil rating: Yes

Galloway

Percent of map unit: 5 percent Landform: Depressions, flats Down-slope shape: Concave, linear Across-slope shape: Concave, linear Hydric soil rating: No

UzC—Udorthents, 0 to 10 percent slopes

Map Unit Setting

National map unit symbol: 1qtkv Elevation: 10 to 200 feet Mean annual precipitation: 42 to 48 inches Mean annual air temperature: 52 to 58 degrees F Frost-free period: 180 to 220 days Farmland classification: Not prime farmland

Map Unit Composition

Udorthents, loamy, and similar soils: 90 percent *Minor components:* 10 percent *Estimates are based on observations, descriptions, and transects of the mapunit.*

Description of Udorthents, Loamy

Setting

Landform: Knolls, flats Down-slope shape: Convex, linear Across-slope shape: Convex, linear Parent material: Fluviomarine sediments

Typical profile

C1 - 0 to 4 inches: sandy loam C2 - 4 to 80 inches: sandy loam

Properties and qualities

Slope: 0 to 10 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Runoff class: Medium
Capacity of the most limiting layer to transmit water (Ksat): High (1.98 to 5.95 in/hr)
Depth to water table: About 40 to 72 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Moderate (about 6.1 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 3e Hydrologic Soil Group: A Hydric soil rating: No

Minor Components

Urban land

Percent of map unit: 10 percent Landform: Flats, knolls Hydric soil rating: No

WHe1—Herring Creek mucky silt loam, 0 to 1 meter water depth

Map Unit Setting

National map unit symbol: 2xhnk Elevation: 0 feet Mean annual precipitation: 41 to 49 inches Mean annual air temperature: 53 to 60 degrees F Frost-free period: 365 days Farmland classification: Not prime farmland

Map Unit Composition

Herring creek, 0 to 1 meter water depth, and similar soils: 85 percent *Minor components:* 15 percent *Estimates are based on observations, descriptions, and transects of the mapunit.*

Description of Herring Creek, 0 To 1 Meter Water Depth

Setting

Landform: Estuarine tidal streams Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Talf Down-slope shape: Concave Across-slope shape: Linear Parent material: Mainland cove fine-silty estuarine deposits over woody organic material

Typical profile

Aseg - 0 to 3 inches: mucky silt loam Cseg - 3 to 24 inches: silt loam Oeseb1 - 24 to 51 inches: mucky peat Oeseb2 - 51 to 69 inches: mucky peat

Properties and qualities

Slope: 0 to 1 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Subaqueous
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.20 to 1.98 in/hr)
Depth to water table: About 0 inches
Frequency of flooding: Very frequent
Frequency of ponding: None
Calcium carbonate, maximum content: 10 percent
Maximum salinity: Strongly saline (16.0 to 35.0 mmhos/cm)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 8 Hydrologic Soil Group: D Hydric soil rating: Yes

Minor Components

Metedeconk, 0 to 1 meter water depth

Percent of map unit: 10 percent Landform: Estuarine tidal streams Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Talf Down-slope shape: Concave Across-slope shape: Linear Hydric soil rating: Yes

Truitt, 0 to 1 meter water depth

Percent of map unit: 5 percent Landform: Mainland coves Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Talf Down-slope shape: Concave Across-slope shape: Concave Hydric soil rating: Yes

References

American Association of State Highway and Transportation Officials (AASHTO). 2004. Standard specifications for transportation materials and methods of sampling and testing. 24th edition.

American Society for Testing and Materials (ASTM). 2005. Standard classification of soils for engineering purposes. ASTM Standard D2487-00.

Cowardin, L.M., V. Carter, F.C. Golet, and E.T. LaRoe. 1979. Classification of wetlands and deep-water habitats of the United States. U.S. Fish and Wildlife Service FWS/OBS-79/31.

Federal Register. July 13, 1994. Changes in hydric soils of the United States.

Federal Register. September 18, 2002. Hydric soils of the United States.

Hurt, G.W., and L.M. Vasilas, editors. Version 6.0, 2006. Field indicators of hydric soils in the United States.

National Research Council. 1995. Wetlands: Characteristics and boundaries.

Soil Survey Division Staff. 1993. Soil survey manual. Soil Conservation Service. U.S. Department of Agriculture Handbook 18. http://www.nrcs.usda.gov/wps/portal/ nrcs/detail/national/soils/?cid=nrcs142p2_054262

Soil Survey Staff. 1999. Soil taxonomy: A basic system of soil classification for making and interpreting soil surveys. 2nd edition. Natural Resources Conservation Service, U.S. Department of Agriculture Handbook 436. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_053577

Soil Survey Staff. 2010. Keys to soil taxonomy. 11th edition. U.S. Department of Agriculture, Natural Resources Conservation Service. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_053580

Tiner, R.W., Jr. 1985. Wetlands of Delaware. U.S. Fish and Wildlife Service and Delaware Department of Natural Resources and Environmental Control, Wetlands Section.

United States Army Corps of Engineers, Environmental Laboratory. 1987. Corps of Engineers wetlands delineation manual. Waterways Experiment Station Technical Report Y-87-1.

United States Department of Agriculture, Natural Resources Conservation Service. National forestry manual. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/ home/?cid=nrcs142p2 053374

United States Department of Agriculture, Natural Resources Conservation Service. National range and pasture handbook. http://www.nrcs.usda.gov/wps/portal/nrcs/ detail/national/landuse/rangepasture/?cid=stelprdb1043084

United States Department of Agriculture, Natural Resources Conservation Service. National soil survey handbook, title 430-VI. http://www.nrcs.usda.gov/wps/portal/ nrcs/detail/soils/scientists/?cid=nrcs142p2_054242

United States Department of Agriculture, Natural Resources Conservation Service. 2006. Land resource regions and major land resource areas of the United States, the Caribbean, and the Pacific Basin. U.S. Department of Agriculture Handbook 296. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/? cid=nrcs142p2_053624

United States Department of Agriculture, Soil Conservation Service. 1961. Land capability classification. U.S. Department of Agriculture Handbook 210. http://www.nrcs.usda.gov/Internet/FSE_DOCUMENTS/nrcs142p2_052290.pdf

Appendix A.4

Time of Concentration Calculation

SCS ENGINEERS

363	ENGINEEKJ	SHEET NO.		1 of 2	2
Job No.	25221174	CALC. NO.		4	
Job:	Indian River Landfill – Run-on/Run-Off Evaluation	REV. NO.		1	
Client	NRG	BY	SJL	DATE	7/10/2021
Subject	Time of Concentration	CHK'D.	RDS	DATE	7/30/2021

Problem Statement

Summarize the time of concentration input parameters for HydroCAD for each subcatchment area. These parameters are used to describe how stormwater runoff is distributed over time. The time of concentration is defined as the longest amount of time that it would take for a drop of water to travel from the headwater of a subcatchment area to its downstream edge (i.e. prior to exiting the subcatchment area and being managed by a downstream element).

<u>Given</u>

The software utilized to model the proposed closure plan conditions of the Landfill is HydroCAD. This program calculates a "weighted" curve number value for each subcatchment based on the percentage of total acreage for each soil type and land cover parameter. This method was utilized in this evaluation and is further described in the "Calculation" section below.

Please find the following supporting information attached to this calculation:

- The time of concentration flow paths for each subcatchment area are shown in Figure A.4-1.
- The methodology that HydroCAD uses to calculate the time of concentration is based on Technical Release (TR) 20 / TR-55, published by the Soil Conservation Service.

Assumptions

The following assumptions were made in the calculations:

- For each subcatchment, the time of concentration (T_c) is the sum of the travel times (T_t) of various consecutive flow segments. Two (2) types of flow are used in the time of concentration calculations: sheet flow and shallow concentrated flow.
- Sheet flow is assumed to become shallow concentrated flow at 100 feet, which is conservative in comparison to 300 feet, which is designated in the TR-55 procedures.
- The Manning's coefficient "n" for sheet flow in grass-covered areas is assumed to be 0.150. This is indicative of short grass cover. During shallow concentrated flow, the average flow velocity is assumed to be 7.0 ft/sec. This is the HydroCAD default for, "Short Grass Pasture" surface cover.
- The Manning's coefficient "n" for sheet flow in paved areas is assumed to be 0.011. This is indicative of smooth surfaces. During shallow concentrated flow, the average flow velocity is assumed to be 20.3 ft/sec. This is the HydroCAD default for, "Paved" surface cover.

SCS	ENGINEERS	SHEET NO.		2 of 2	
Job No.	25221174	CALC. NO.		4	
Job:	Indian River Landfill – Run-on/Run-Off Evaluation	REV. NO.		1	
Client	NRG	BY	SJL	DATE	7/10/2021
Subject	Time of Concentration	CHK'D.	RDS	DATE	7/30/2021

Calculations

The following formulas are used by HydroCAD to determine lag times: Sheet Flow:

Sheet flow is flow over plane surfaces and is calculated by HydroCAD using the following equation.

$$T_{t} = \frac{(0.007(nL)^{0.8})}{P_{2}^{0.5} S^{0.4}}$$

Where:

- $T_t = Travel time (hours)$
- $P_2 = 2$ -year, 24-hour rainfall depth
- S = Land slope along flow path (ft/ft)
- L = Flow Length (ft)
- n = Manning's coefficient

Shallow Concentrated Flow:

Shallow concentrate flow time is calculated by HydroCAD using the following equation.

$$\begin{split} T_t &= \frac{L}{3,600V} \quad \text{where} \quad V = K_v \sqrt{s} \\ T_t &= \text{Travel Time (hours)} \\ L &= \text{Flow Length (ft)} \\ V &= \text{Average Velocity (ft/sec)} \\ 3,600 &= \text{Conversion factor from seconds to hours} \\ K_v &= \text{Velocity Factor (ft/sec)} \\ s &= \text{Land Slope (ft/sec)} \end{split}$$

Results

A summary of the flow lengths, slopes, and other key parameters used to calculate the time of concentration for each subcatchment area is provided in attached **Table A.4-1**. The table also includes the time of concentration values calculated by HydroCAD for each subcatchment area. Refer to **Appendix A.8** for a copy of the HydroCAD summary pages which lists the parameters entered in the stormwater model.

Indian River Landfill

Table A.4-1 Substathment Time of Concentration Summary								
		300	Catchinient Time of	t Elow	Shallow Con	Time of		
Area	Area (Acres)	Curve Number	Janeth (ft)	Class (6)(6)	Sildilow Colli		Concentration (Min)	
SC-A1	0.74	39	100	0.035	Lengur (it)	Siope (IVII)	(Will) 7.6	
SC-A2	0.39	39	100	0.035	84	0.035	8.7	
SC-A3	0.81	39	94	0.181			37	
SC-44	0.80	39	62	0.333	_	_	2.1	
SC-45	0.00	30	54	0.333			1.0	
50.46	0.46	70	61	0.000			1.5	
SC 47	0.40	20	72	0.098	-	-	0.4	
SC-A7	0.27	39	12	0.333	-	-	2.4	
50-A0	0.50	39	40 E0	0.333		-	1.7	
50-A9	0.08	39	50	0.333	-	-	1.0	
SC-A10	0.37	45	60	0.231	-	-	2.5	
SC-A11	0.72	39	47	0.333	-	-	1.7	
SC-A12	0.17	51	12	0.017	•	-	1.9	
SC-B1	0.43	39	100	0.035	83	0.035	8.7	
SC-B2	0.89	39	100	0.035	75	0.035	8.6	
SC-B3	3.16	39	100	0.005	325	0.032	20.9	
SC-B4	1.18	39	100	0.005	365	0.024	22.2	
SC-B5	1.27	39	100	0.078	10	0.333	5.6	
SC-B6	0.03	39	43	0.333	-	-	1.6	
SC-B7	0.46	39	50	0.333	-	-	1.8	
SC-B8	0.53	70	58	0.069	-	-	0.5	
SC-B9	0.13	39	63	0.333	-	-	2.1	
SC-B10	0.48	40	46	0.333	-	-	1.7	
SC-B11	0.02	53	17	0.005	-	-	4.0	
SC-B12	1.00	39	60	0.333	-	-	2.1	
SC-B13	0.28	39	61	0.333	-	-	2.1	
SC-B14	1.11	39	58	0.333	-	-	2.0	
SC-B15	0.59	39	57	0.333	-	-	2.0	
SC-B16	0.33	39	24	0.333	-	-	1.0	
SC-B17	0.16	51	9	0.125	-	-	0.7	
SC-C1	0.42	39	62	0.108	-	-	3.3	
SC-C2	0.04	39	17	0.333		-	0.7	
SC-C3	0.55	39	59	0.333	-	-	2.0	
SC-C4	0.31	39	59	0.333	-	-	2.0	
SC-C5	0.52	39	59	0.333	-	_	2.0	
SC-C6	0.31	39	59	0.333	-	-	2.0	
SC-C7	0.48	39	58	0.333	_	_	2.0	
SC-C8	0.40	39	58	0.000			2.0	
SC-09	0.00	30	12	0.000	_	_	0.6	
SC-03	0.00	55	 0	0.333	_	_	0.0	
20-010 20-010	1 70	20	9 100	0.005	-	-	0.4	
	1./8	39	100	0.005	200	0.030	20.1	
30-D2	0.10	39	100	0.035	143	0.035	9.4	
SC-D3	1.13	39	100	0.035	193	0.135	8.8	
SC-D4	1.01	39	61	0.333	-	-	2.1	
SC-D5	1.05	39	57	0.333	-	-	2.0	
SC-D6	0.14	39	15	0.333	-	-	0.7	
SC-D7	0.07	50	8	0.333	-	-	0.4	
SC-E1	5.74	39	100	0.014	643	0.013	24.3	
SC-E2	0.15	39	55	0.221	-	-	2.3	
SC-E3	0.31	39	63	0.333	-	-	2.1	
SC-E4	0.39	39	56	0.333	-	-	1.9	
SC-E5	0.46	39	72	0.333	-	-	2.4	



Indian River Landfill

Table A.4-1 Subcatchment Time of Concentration Summary								
Subatahmant			Shee	t Flow	Shallow Conc	Time of		
Area (Acres)		Curve Number	Sileet Flow		Length (ff)	Concentration (Min)		
SC-E6	0.27	63	57	0.070		-	0.5	
SC-E7	0.22	39	47	0.333	-	-	1.7	
SC-E8	0.12	58	23	0.333		-	0.1	
SC-F1	0.97	39	71	0.239	-	-	2.7	
SC-F2	1.22	39	63	0.333	-	-	2.1	
SC-F3	1.36	39	63	0.333	-	-	2.1	
SC-E4	0.85	39	47	0.333		_	17	
SC-E5	0.43	63	75	0.059		_	0.6	
SC-61	0.59	39	100	0.027	133	0.137	9.3	
SC-62	1 10	39	100	0.043	240	0.057	9.4	
60 G2	0.42	20	62	0.222	240	0.001	0.1	
30-d3	0.43	39	61	0.333	-	-	2.1	
50-64	0.40	39	62	0.333	-	-	2.1	
50-05	0.44	39	63	0.333	-	-	2.1	
SC-G6	0.40	39	61	0.333	-	-	2.1	
SC-G7	0.22	39	43	0.333	-	-	1.6	
SC-G8	0.10	63	19	0.632	-	-	0.1	
SC-G9	0.81	39	64	0.333	-	-	2.2	
SC-G10	0.27	62	19	0.106	-	-	0.2	
SC-H1	0.54	39	60	0.333	-	-	2.1	
SC-H2	0.78	39	100	0.333	-	-	3.1	
SC-H3	0.82	39	61	0.333	-	-	2.1	
SC-H4	0.71	39	51	0.333	-	-	1.8	
SC-H5	0.98	39	61	0.333	-	-	2.1	
SC-H6	0.93	39	55	0.333	-	-	1.9	
SC-H7	0.40	39	61	0.333	-	-	2.1	
SC-H8	0.18	66	27	0.037	-	-	2.6	
SC-H9	0.71	39	66	0.333	-	-	2.2	
SC-H10	0.44	68	31	0.048	-	-	2.6	
SC-I1	1.91	39	100	0.043	388	0.015	14.6	
SC-I2	0.54	39	91	0.333	-	-	2.9	
SC-I3	0.18	39	50	0.333	-	-	1.8	
SC-14	0.06	39	54	0.333	-	-	1.9	
SC-15	0.40	39	74	0.333	-	-	2.4	
SC-16	0.16	63	28	0.170	-	-	0.2	
SC-J1	2.81	39	100	0.023	484	0.017	17.8	
SC-J2	0.14	39	78	0.333	-	-	2.5	
SC-J3	0.29	39	61	0.333	-	-	2.1	
SC-J4	0.40	39	63	0.333	-	-	2.1	
SC-J5	0.42	39	62	0.333	-	-	2.1	
SC-J6	0.24	65	39	0.128	-	-	0.3	
SC-K1	2.80	39	100	0.010	472	0.019	20.7	
SC-K2	1.40	39	100	0.035	156	0.035	9.6	
SC-K3	0.37	39	38	0.333	-	-	1.4	
SC-K4	0.66	39	89	0.035	-	-	6.9	
SC-K5	0.69	39	61	0.333	-	-	2.1	
SC-K6	1.11	39	100	0.125	25	0.333	4.7	
SC-K7	0.63	39	61	0.333	-	-	2.1	
SC-K8	1.09	39	61	0.333	-	-	2.1	
SC-K9	0.57	39	61	0.333	-	-	2.1	
SC-K10	0.94	39	60	0.333	-	-	2.1	
SC-K11	0.29	39	51	0.333	-	-	1.8	


Table A.4-1 Subcatchment Time of Concentration Summary							
Subcatchment	Area (Acres)	Curve Number	Sheet Flow		Sheet Flow Shallow Concentrated Flow		Time of Concentration
Area	· · /		Length (ft)	Slope (ft/ft)	Length (ft)	Slope (ft/ft)	(Min)
SC-K12	0.15	64	41	0.146	-	-	0.3
SC-K13	0.41	39	46	0.333	-	-	1.7
SC-K14	0.25	69	35	0.092	-	-	0.3
SC-NE1	0.32	42	41	0.333	-	-	1.5
SC-NE2	0.50	98		Diminimus travel time due to direct precipitation.			
SC-NE3	0.47	49	78	0.141	-	-	3.6
SC-NE4	0.46	98		Diminimus tra	vel time due to direc	t precipitation.	
SC-NE5	8.05	40	94	0.202	-	-	3.6
SC-SW1	0.17	39	41	0.244	-	-	1.7
SC-SW2	0.40	98	Diminimus travel time due to direct precipitation.				
SC-SW3	0.45	39	15	0.200	-	-	0.8
SC-SW4	3.20	98		Diminimus travel time due to direct precipitation.			







0 G	300' RAPHIC SCALE
L	EGEND
· _ · _ · _ · _ · _	APPROXIMATE PHASE I PERMITTED WASTE BOUNDARY
	APPROXIMATE PHASE II PERMITTED WASTE BOUNDARY
	EXISTING CONTOUR OUTSIDE LANDFILL FOOTPRINT
	EXISTING/PROPOSED CONTOUR WITHIN LANDFILL FOOTPRINT
	OUTFALL LOCATION
<u> </u>	SUBCATCHMENT AREA BOUNDARY
$\rightarrow \rightarrow \rightarrow \rightarrow \rightarrow \rightarrow \rightarrow$	TIME OF CONCENTRATION FLOWPATH

NOTES

1. EXISTING TOPOGRAPHY OF THE SURFACE WATER SEDIMENT CONTROL BASIN IS FROM FILE "09-034 topo.dwg," PREPARED BY SOULE AND ASSOCIATES, P.C., DATED MARCH 2009. ALL OTHER EXISTING TOPOGRAPHY PROVIDED BY WENCK ASSOCIATES, INC., 1800 PIONEER CREEK CENTER, MAPLE PLAIN MN. 55359. DATE OF AERIAL PHOTOGRAPHY: JANUARY 9, 2003.

FIGURE A4-1	FIGURE
TIME OF CONCENTRATION FLOWPAATH	3 OF 4

Appendix A.5

Subcatchment Area Discharge Rates

|--|

SCS	ENGINEERS	SHEET NO.		1 of 1	<u> </u>
Job No.	25221174	CALC. NO.		5	
Job:	Indian River Landfill – Run-on/Run-Off Evaluation	REV. NO.		1	
Client	NRG	BY	SJL	DATE	7/29/2021
Subject	Subcatchment Discharge Rates	CHK'D.	ZPC	DATE	7/30/2021

Problem Statement

Determine the stormwater runoff rates for the each subcatchment area

Given

Parameters, such as rainfall, acreage, curve number, and flow length discussed in previous sections of this appendix are entered into HydroCAD for each subcatchment. The stormwater discharge rates for each subcatchment are calculated in HydroCAD using these parameters. This calculation sheet provides a summary of the input values and the HydroCAD model results. Equations to determine these parameters are described in previous portions of Appendix A.

Storm Model Setup

The stormwater methodology and base information was defined as follows:

Runoff Calculation Method:	SCS TR-20
Reach Routing Method:	Storage Indication Plus Translation Method
Pond Routing Method:	Storage Indication Method (Modified-Plus)
Storm Distribution:	NRCS (SCS) Type III, 24-hour
Unit Hydrograph:	SCS
Antecedent Moisture Condition:	2

The Natural Resources Conservation Service (NRCS) developed methods TR-20 and TR-55 as standardized stormwater modeling. Both provide similar results. TR-20 is the computer-based modeling approach that is more complex and generally considered slightly more accurate than TR-55. TR-55, frequently called the tabular method, was developed after TR-20 to help simplify the modeling process. TR-55 was developed to use chart-based solutions with the SCS runoff equation. For the purpose of this hydrologic model, TR-20 methodology was used.

Model Calculations and Results

The stormwater model was analyzed for the 25-year, 24-hour storm event. A summary of the discharge rates for each subcatchment area is provided in Table A.5-1. In addition, reports summarizing the results of the HydroCAD model runs are provided in Appendix A.8.

Table A.5-1 Subcatchment Discharge Rate Summary					
Subsetshment Area Area (Area) 25-Year, 24-Hour Storm Duration					
Subcatchment Area	Area (Acres)	cubic feet per second (cfs)			
SC-A1	0.74	0.45			
SC-A2	0.39	0.23			
SC-A3	0.81	0.63			
SC-A4	0.80	0.71			
SC-A5	0.43	0.38			
SC-A6	0.46	3.14			
SC-A7	0.27	0.24			
SC-A8	0.50	0.45			
SC-A9	0.68	0.61			
SC-A10	0.37	0.70			
SC-A11	0.72	0.65			
SC-A12	0.17	0.51			
SC-B1	0.43	0.26			
SC-B2	0.89	0.54			
SC-B3	3.16	1.10			
SC-B4	1.18	0.40			
SC-B5	1.27	0.93			
SC-B6	0.03	0.03			
SC-B7	0.46	0.41			
SC-B8	0.53	3.64			
SC-B9	0.13	0.11			
SC-B10	0.48	0.51			
SC-B11	0.02	0.06			
SC-B12	1.00	0.89			
SC-B13	0.28	0.24			
SC-B14	1.11	0.99			
SC-B15	0.59	0.52			
SC-B16	0.33	0.29			
SC-B17	0.16	0.49			
SC-C1	0.42	0.34			
SC-C2	0.04	0.03			
SC-C3	0.55	0.49			
SC-C4	0.31	0.28			
SC-C5	0.52	0.47			
SC-C6	0.31	0.28			
SC-C7	0.48	0.43			
SC-C8	0.30	0.27			
SC-C9	0.06	0.06			
SC-C10	0.05	0.16			
SC-D1	1.78	0.64			
SC-D2	0.10	0.06			
SC-D3	1.13	0.67			
SC-D4	1.01	0.90			
SC-D5	1.05	0.94			
SC-D6	0.14	0.13			
SC-D7	0.07	0.22			





Table A.5-1 Subcatchment Discharge Rate Summary					
Subcatchment Area	Area (Acres)	25-Year, 24-Hour Storm Duration			
Subcatchment Area		cubic feet per second (cfs)			
SC-E1	5.74	1.80			
SC-E2	0.15	0.13			
SC-E3	0.31	0.28			
SC-E4	0.39	0.34			
SC-E5	0.46	0.40			
SC-E6	0.27	1.47			
SC-E7	0.22	0.20			
SC-E8	0.12	0.52			
SC-F1	0.97	0.83			
SC-F2	1.22	1.08			
SC-F3	1.36	1.21			
SC-F4	0.85	0.76			
SC-F5	0.43	2.35			
SC-G1	0.59	0.34			
SC-G2	1.10	0.64			
SC-G3	0.43	0.38			
SC-G4	0.40	0.36			
SC-G5	0.44	0.39			
SC-G6	0.40	0.36			
SC-G7	0.22	0.20			
SC-G8	0.10	0.57			
SC-G9	0.81	0.72			
SC-G10	0.27	1.43			
SC-H1	0.54	0.48			
SC-H2	0.78	0.64			
SC-H3	0.82	0.73			
SC-H4	0.71	0.64			
SC-H5	0.98	0.87			
SC-H6	0.93	0.83			
SC-H7	0.40	0.35			
SC-H8	0.18	1.01			
SC-H9	0.71	0.62			
SC-H10	0.44	2.71			
SC-I1	1.91	0.84			
SC-12	0.54	0.46			
SC-I3	0.18	0.16			
SC-14	0.06	0.05			
SC-15	0.40	0.35			
SC-16	0.16	0.86			
SC-J1	2.81	1.09			
SC-J2	0.14	0.12			
SC-J3	0.29	0.26			
SC-J4	0.40	0.35			
SC-J5	0.42	0.37			
SC-J6	0.24	1.43			
SC-K1	2.80	0.98			





Table A.5-1 Subcatchment Discharge Rate Summary					
Subcatchment Area	Area (Acres)	25-Year, 24-Hour Storm Duration			
	,	cubic feet per second (cfs)			
SC-K2	1.40	0.80			
SC-K3	0.37	0.33			
SC-K4	0.66	0.44			
SC-K5	0.69	0.61			
SC-K6	1.11	0.84			
SC-K7	0.63	0.56			
SC-K8	1.09	0.97			
SC-K9	0.57	0.51			
SC-K10	0.94	0.84			
SC-K11	0.29	0.26			
SC-K12	0.15	0.85			
SC-K13	0.41	0.37			
SC-K14	0.25	1.66			
SC-NE1	0.32	0.45			
SC-NE2	0.50	5.35			
SC-NE3	0.47	1.19			
SC-NE4	0.46	4.95			
SC-NE5	8.05	7.68			
SC-SW1	0.17	0.15			
SC-SW2	0.40	4.26			
SC-SW3	0.45	0.40			
SC-SW4	3.20	2.87			



Appendix A.6

Landfill Conveyance Feature Sizing

SCS ENGINEERS		SHEET NO.		1 of 3		
Job No.	25221174	CALC. NO.		6		
Job:	Indian River Landfill – Run-on/Run-Off Evaluation	REV. NO.		1		
Client	NRG	BY	SJL	DATE	7/29,	
Subject	Landfill Conveyance Feature Sizing	CHK'D.	RDS	DATE	7/30,	

Problem Statement

Determine whether the stormwater conveyance features that collect and convey stormwater from the Landfill are designed to convey the 25-year, 24-hour storm event without causing erosion or overtopping.

<u>2021</u> 2021

<u>Given</u>

- Stormwater conveyance features include the following:
 - Terrace berms
 - o Letdown pipes
 - Perimeter ditches
 - o Culverts
- The locations of the stormwater conveyance features for the Landfill are shown on **Figure A.6-1.**
- The design summary of the stormwater conveyance features for the Landfill are provided in the following attached tables:
 - A.6-1 Terrace Berm Design Summary
 - A.6-2 Letdown Pipe Design Summary
 - A.6-3 Perimeter Ditch Design Summary
 - A.6-4 Culvert Design Summary
- Discharge rates for each subcatchment area are provided in Appendix A.5.

Design Assumptions

Terrace Berms

- Terrace berms on the Landfill consist of a variety of lining materials, depths, sideslopes, bottom widths, and channel slopes. See **Table A.6-1** for these parameters associated with each terrace berm.
- The following Manning's coefficients were modeled to represent the corresponding terrace berm lining:
 - Manning's coefficient = 0.030 = grass
 - Manning's coefficient = 0.035 = riprap
- Terrace berms with identified flow velocities greater than 5 feet per second (fps) in grasslined berms will be lined with riprap or other approved erosion control material. Tractive forces (shear stress) acting along the terrace bed and sideslope lining shall not exceed the maximum allowable shearing resistance for the selected lining material. It is assumed that all terraces with a flow velocity less than 5 fps meet this criterion and are not anticipated to require erosion control lining material.
- Terrace berms shall convey run-off from the all modeled storm events without overtopping.

SCS	ENGINEERS	SHEET NO.		2 of 3
Job No.	25221174	CALC. NO.		6
Job:	Indian River Landfill – Run-on/Run-Off Evaluation	REV. NO.		1
Client	NRG	BY	SJL	DATE 7/29/2021
Subject	Landfill Conveyance Feature Sizing	CHK'D.	RDS	DATE 7/30/2021

Letdown Pipes

- Letdown pipes consist of one (1) 24-inch high-density polyethylene pipe. At each terrace berm intersection, the letdown pipe is equipped with three (3) 24-inch inlets to collect stormwater into the single, 24-inch letdown pipe. This inlet structure allows for the terrace berm to flow into the downchute pipe without having a localized depression at the inlet.
- A Manning's coefficient of 0.013 was modeled in HydroCAD to represent corrugated polyethylene pipe with smooth interior. See **Table A.6-2** for specific channel slopes of each letdown pipe.
- Letdown pipes shall convey run-off from the modeled storm events without overtopping or backing up.

Perimeter Ditches

- Perimeter ditch segments are constructed and lined with cable concrete channel lining. Each perimeter ditch has varying depths, sideslopes, bottom widths, and channel slopes. See **Table A.6-3** for these parameters associated with each perimeter ditch segment.
- A Manning's coefficient of 0.013 was modeled in HydroCAD to represent concrete channel lining.

Culverts

- Two (2) total culvert locations are used to convey stormwater from the Landfill and have all been constructed.
- Multiple culverts are located at culvert locations C-1 and C-2. The maximum allowable discharge of each culvert is multiplied by the number of culverts when determining its ability to convey each storm event.
- Each culvert location consists of varying pipe diameters and slopes. See **Table A.6-4** for these parameters associated with each perimeter ditch segment.
- A Manning's coefficient of 0.012 was modeled in HydroCAD to represent concrete pipes.
- A Manning's coefficient of 0.025 was modeled in HydroCAD to represent corrugated metal pipes.

Calculations and Results

The model results and sizing analysis of the stormwater conveyance features serving the Landfill are provided in the following attached tables:

- A.6-1 Terrace Berm Design Summary
- A.6-2 Letdown Pipe Design Summary
- A.6-3 Perimeter Ditch Design Summary
- A.6-4 Culvert Design Summary

SCS	ENGINEERS	SHEET NO.		3 of 3	3
Job No.	25221174	CALC. NO.		6	
Job:	Indian River Landfill – Run-on/Run-Off Evaluation	REV. NO.		1	
Client	NRG	BY	SJL	DATE	7/29/2021
Subject	Landfill Conveyance Feature Sizing	CHK'D.	RDS	DATE	7/30/2021

The following key points are noted:

- All features will convey the stormwater associated with the 10-year and 100-year, 24-hour storm without overtopping, including:
 - o Terrace Berms;
 - o Letdown Pipes;
 - Perimeter Ditches; and
 - o Culverts.
- All grass-lined terrace berms are able to convey the 25-year, 24-hour storm with flow velocities less than 5 fps.
- Outlet protection is or will be provided at the interface between all letdown pipes to perimeter ditch segments, and culvert outlet areas. Outlet protection will consist of riprap or other approved erosion control lining material placed at the outlet location to reduce exit flow velocities and to minimize erosion and scour due to flow velocities exceeding 5 fps.

	Table A.6-1 Terrace Berm Design Summary									
	Design Parameters Model Results									
Terrace Berm Identifier	Left Sideslope	Right Sideslope	Slope	Depth	25-Year Storm Peak Velocity	Peak Depth	Design Depth > Peak Depth?	Flow Rate < 5 fps?	Erosion Control Anticipated to be Required?	
	(H:1V)	(H:1V)	(ft/ft)	(ft)	(ft/sec)	(ft)	YES/NO	YES/NO	YES/NO	
TB-A1	28	1	0.0101	2.00	0.91	0.16	YES	YES	NO	
TB-A2	28	1	0.008	2.00	0.73	0.13	YES	YES	NO	
TB-A3	10	3	0.019	1.20	1.51	0.21	YES	YES	NO	
TB-A4	10	3	0.017	1.20	1.43	0.21	YES	YES	NO	
TB-A5	2	3	0.039	2.00	3.49	0.58	YES	YES	NO	
TB-A6	10	3	0.023	1.20	1.32	0.15	YES	YES	NO	
TB-A7	10	3	0.018	1.20	1.23	0.17	YES	YES	NO	
TB-A8	10	3	0.013	1.20	1.22	0.20	YES	YES	NO	
TB-A9	10	3	0.016	1.20	1.48	0.24	YES	YES	NO	
TB-B1	28	1	0.008	2.00	0.73	0.14	YES	YES	NO	
TB-B2	28	1	0.009	2.00	0.90	0.17	YES	YES	NO	
TB-B3	28	1	0.006	2.00	0.98	0.25	YES	YES	NO	
TB-B4	28	1	0.013	2.00	1.04	0.16	YES	YES	NO	
TB-B5	10	3	0.017	1.20	1.90	0.33	YES	YES	NO	
TB-B6	10	3	0.020	1.20	0.76	0.07	YES	YES	NO	
TB-B7	2	3	0.035	2.00	3.41	0.61	YES	YES	NO	
TB-B8	10	3	0.020	1.20	1.06	0.12	YES	YES	NO	
TB-B9	10	3	0.017	1.20	1.29	0.18	YES	YES	NO	
TB-B10	10	3	0.001	1.20	0.31	0.17	YES	YES	NO	
TB-B11	10	3	0.014	1.20	1.33	0.23	YES	YES	NO	
TB-B12	10	3	0.010	1.20	0.95	0.17	YES	YES	NO	
TB-B13	10	3	0.014	1.20	1.34	0.23	YES	YES	NO	
TB-B14	10	3	0.013	1.20	1.20	0.20	YES	YES	NO	
TB-C1	10	3	0.008	1.20	0.89	0.19	YES	YES	NO	
TB-C2	10	3	0.011	1.20	0.60	0.08	YES	YES	NO	
TB-C3	10	3	0.012	1.20	1.19	0.20	YES	YES	NO	
TB-C4	10	3	0.011	1.20	1.01	0.18	YES	YES	NO	
TB-C5	10	3	0.010	1.20	1.06	0.21	YES	YES	NO	
TB-C6	10	3	0.010	1.20	1.00	0.18	YES	YES	NO	
TB-C7	10	3	0.010	1.20	1.08	0.20	YES	YES	NO	
TB-C8	10	3	0.009	1.20	0.94	0.18	YES	YES	NO	



	Table A.6-1 Terrace Berm Design Summary									
		Design Pa	rameters				Model Result	s		
Terrace Berm Identifier	Left Sideslope	Right Sideslope	Slope	Depth	25-Year Storm Peak Velocity	Peak Depth	Design Depth > Peak Depth?	Flow Rate < 5 fps?	Erosion Control Anticipated to be Required?	
	(H:1V)	(H:1V)	(ft/ft)	(ft)	(ft/sec)	(ft)	YES/NO	YES/NO	YES/NO	
TB-D1	28	1	0.000	2.00	0.31	0.33	YES	YES	NO	
TB-D2	28	1	0.006	2.00	0.49	0.09	YES	YES	NO	
TB-D3	10	3	0.016	1.20	1.40	0.22	YES	YES	NO	
TB-D4	10	3	0.016	1.20	1.42	0.23	YES	YES	NO	
TB-D5	10	3	0.015	1.20	1.40	0.23	YES	YES	NO	
TB-E1	28	1	0.014	2.00	0.76	0.10	YES	YES	NO	
TB-E2	28	1	0.017	2.00	0.96	0.12	YES	YES	NO	
TB-E3	28	1	0.015	2.00	0.94	0.13	YES	YES	NO	
TB-F1	28	1	0.014	2.00	1.05	0.15	YES	YES	NO	
TB-F2	28	1	0.015	2.00	1.14	0.16	YES	YES	NO	
TB-F3	28	1	0.016	2.00	1.18	0.16	YES	YES	NO	
TB-G1	28	1	0.015	2.00	1.01	0.14	YES	YES	NO	
TB-G2	28	1	0.014	2.00	1.15	0.18	YES	YES	NO	
TB-G3	28	1	0.015	2.00	0.99	0.13	YES	YES	NO	
TB-G4	28	1	0.014	2.00	0.95	0.13	YES	YES	NO	
TB-G5	28	1	0.015	2.00	0.99	0.14	YES	YES	NO	
TB-G6	28	1	0.014	2.00	0.95	0.13	YES	YES	NO	
TB-H1	28	1	0.015	2.00	0.96	0.13	YES	YES	NO	
TB-H2	28	1	0.020	2.00	1.23	0.15	YES	YES	NO	
TB-H3	28	1	0.019	2.00	1.18	0.15	YES	YES	NO	
TB-H4	28	1	0.018	2.00	1.11	0.14	YES	YES	NO	
TB-H5	28	1	0.018	2.00	1.19	0.15	YES	YES	NO	
TB-H6	28	1	0.018	2.00	1.16	0.15	YES	YES	NO	
TB-I1	28	1	0.016	2.00	1.06	0.15	YES	YES	NO	
TB-I2	28	1	0.003	2.00	0.43	0.13	YES	YES	NO	
TB-I3	28	1	0.024	2.00	0.79	0.07	YES	YES	NO	
TB-J1	28	1	0.014	2.00	0.79	0.10	YES	YES	NO	
TB-J2	28	1	0.011	2.00	0.81	0.13	YES	YES	NO	
TB-J3	28	1	0.014	2.00	0.94	0.13	YES	YES	NO	
TB-K1	28	1	0.010	2.00	1.04	0.20	YES	YES	NO	
TB-K2	10	3	0.016	1.20	1.12	0.16	YES	YES	NO	
TB-K3	10	3	0.016	1.20	1.30	0.19	YES	YES	NO	
TB-K4	10	3	0.014	1.20	1.30	0.21	YES	YES	NO	
TB-K5	2	3	0.016	2.00	1.59	0.36	YES	YES	NO	
TB-K6	10	3	0.016	1.20	1.36	0.21	YES	YES	NO	
TB-K7	10	3	0.016	1.20	1.42	0.23	YES	YES	NO	
TB-K8	10	3	0.014	1.20	1.27	0.20	YES	YES	NO	
TB-K9	10	3	0.013	1.20	1.30	0.23	YES	YES	NO	
TB-K10	2	3	0.031	2.00	2.77	0.48	YES	YES	NO	



Table A.6-2 Letdown Pipe Design Summary								
	Design Pa	arameters		Model Results				
Letdown Pipe Identifier	Slope	Diameter	25-Year Storm	Peak Depth	Design Depth >			
	(ft/ft)	(ft)	(ft/sec)	(ft)	YES/NO			
LP-1A	0.2386	2	8.9	0.10	YES			
LP-1B	0.2152	2	10.0	0.13	YES			
LP-1C	0.2497	2	11.7	0.14	YES			
LP-1D	0.0186	2	6.3	0.43	YES			
LP-1E	0.1629	2	13.5	0.25	YES			
LP-1F	0.2823	2	17.1	0.24	YES			
LP-2A	0.2186	2	8.9	0.1	YES			
LP-2B	0.2175	2	12.1	0.17	YES			
LP-2C	0.0375	2	8.1	0.37	YES			
LP-2D	0.1782	2	14.1	0.25	YES			
LP-2E	0.2073	2	15.2	0.25	YES			
LP-2F	0.1426	2	13.6	0.29	YES			
LP-3A	0.2832	2	7.2	0.06	YES			
LP-3B	0.2698	2	9.9	0.1	YES			
LP-3C	0.2465	2	11.3	0.14	YES			
LP-3D	0.0622	2	7.7	0.22	YES			
LP-4A	0.2424	2	8.9	0.1	YES			
LP-4B	0.2710	2	10.5	0.11	YES			
LP-4C	0.2465	2	10.9	0.13	YES			
LP-4D	0.0706	2	7.6	0.2	YES			
LP-5A	0.3331	2	14.7	0.16	YES			
LP-5B	0.2723	2	13.8	0.17	YES			
LP-5C	0.2654	2	13.8	0.18	YES			
LP-5D	0.2340	2	13.4	0.19	YES			



Table A.6-2 Letdown Pipe Design Summary							
	Design Pa	arameters		Model Results			
Letdown Pipe Identifier	Slope	Diameter	25-Year Storm Peak Velocity	Peak Depth	Design Depth > Peak Depth?		
	(ft/ft)	(ft)	(ft/sec)	(ft)	YES/NO		
LP-6A	0.1909	2	7.3	0.09	YES		
LP-6B	0.1855	2	9.3	0.13	YES		
LP-6C	0.1688	2	10.3	0.16	YES		
LP-7A	0.2774	2	10.6	0.11	YES		
LP-7B	0.2710	2	11.8	0.14	YES		
LP-7C	0.2406	2	12.3	0.16	YES		
LP-8A	0.1795	2	6.4	0.07	YES		
LP-8B	0.1351	2	7.8	0.12	YES		
LP-8C	0.1483	2	9.2	0.15	YES		
LP-8D	0.2292	2	11.6	0.15	YES		
LP-8E	0.2203	2	12.4	0.18	YES		
LP-8F	0.1298	2	10.9	0.22	YES		
LP-9A	0.3254	2	11.5	0.12	YES		
LP-9B	0.2394	2	11.3	0.14	YES		
LP-9C	0.2937	2	12.4	0.14	YES		
LP-9D	0.2626	2	12.0	0.14	YES		
LP-10A	0.2729	2	11.7	0.14	YES		
LP-10B	0.2690	2	11.8	0.14	YES		
LP-10C	0.2742	2	12.1	0.14	YES		
LP-10D	0.3047	2	13.1	0.15	YES		
LP-11A	0.1951	2	11.8	0.18	YES		
LP-11B	0.2714	2	14.2	0.18	YES		
LP-11C	0.2663	2	15.4	0.21	YES		
LP-11D	0.2523	2	16.1	0.24	YES		
LP-11E	0.3240	2	18.4	0.24	YES		



	Table A.6-3 Perimeter Ditch Design Summary											
			D	esign Paramete	rs				Model Res	ults		
Ditch Name	Left Sideslope	Right Sideslope	Slope	Depth	Base Width	Manning's Coefficient	Lining Material	25-Year Storm Peak Velocity	Peak Depth	Design Depth > Peak Depth?		
	(H:1V)	(H:1V)	(ft/ft)	(ft)	(ft)	(n)	-	(ft/sec)	(ft)	YES/NO		
PC-1	1	1	0.0146	1.0	8	0.013	Concrete	1.15	0.02	YES		
PC-2	1	1	0.0080	1.0	8	0.013	Concrete	2.49	0.12	YES		
PC-3	1	1	0.0113	1.0	8	0.013	Concrete	3.85	0.19	YES		
PC-4	2	3	0.0099	2.0	6	0.013	Concrete	3.94	0.22	YES		
PC-5	2	3	0.0201	2.0	2	0.013	Concrete	4.68	0.19	YES		
PC-6	2	3	0.0064	2.0	6	0.013	Concrete	3.75	0.30	YES		
PC-7	2	3	0.0041	2.0	4	0.013	Concrete	3.29	0.37	YES		
PC-8	2	3	0.0057	2.0	2	0.013	Concrete	3.84	0.41	YES		
PC-9	2	3	0.0028	2.0	2	0.013	Concrete	2.64	0.39	YES		
PC-10	2	3	0.0148	2.0	2	0.013	Concrete	4.27	0.21	YES		
PC-11	2	3	0.0160	2.0	2	0.013	Concrete	2.86	0.10	YES		
PC-12	2	3	0.0104	2.0	2	0.013	Concrete	3.83	0.24	YES		
PC-13	2	3	0.0160	2.0	2	0.013	Concrete	4.40	0.21	YES		
PC-14	2	3	0.0214	2.0	2	0.013	Concrete	3.10	0.09	YES		



	Table A.6-4 Culvert Design Summary										
		Des	ign Parameters			Model Results					
Culvert Location Identifier	Diameter (in)	Slope (%)	Pipe Material	Number of Culverts at Location	25-Year Storm Peak Velocity (ft/sec)	Peak Depth (in)	Design Diameter > Peak Depth? YES/NO				
C-1	24	0.30	Corrugated Metal Pipe	2	6.75	13.32	YES				
C-2	30	9.85	Concrete Pipe	2	12.56	3.48	YES				







0 G	300' RAPHIC SCALE
Ĺ	EGEND
	APPROXIMATE PHASE I PERMITTED WASTE BOUNDARY
	APPROXIMATE PHASE II PERMITTED WASTE BOUNDARY
-10	EXISTING CONTOUR OUTSIDE LANDFILL FOOTPRINT
-90	EXISTING/PROPOSED CONTOUR WITHIN LANDFILL FOOTPRINT
	OUTFALL LOCATION
	SUBCATCHMENT AREA BOUNDARY
 <	EXISTING CULVERT LOCATION
→ - → - → - → - → - → - → - → - → - → -	TERRACE BERM LOCATION
$\rightarrow \rightarrow \rightarrow$	LETDOWN PIPE LOCATION
~~~~~	PERIMETER DITCH LOCATION

NOTES

1. EXISTING TOPOGRAPHY OF THE SURFACE WATER SEDIMENT CONTROL BASIN IS FROM FILE "09-034 topo.dwg," PREPARED BY SOULE AND ASSOCIATES, P.C., DATED MARCH 2009. ALL OTHER EXISTING TOPOGRAPHY PROVIDED BY WENCK ASSOCIATES, INC., 1800 PIONEER CREEK CENTER, MAPLE PLAIN MN. 55359. DATE OF AERIAL PHOTOGRAPHY: JANUARY 9, 2003.

FIGURE A.6-1	FIGURE
STORMWATER CONVEYANCE FEATURES	4 OF 4

Appendix A.7

Detention Basin Sizing

AC ENGLARED

363	ENGINEERS	SHEET NO.		1 of 1	
Job No.	25221174	CALC. NO.		7	
Job:	Indian River Landfill – Run-on/Run-Off Evaluation	REV. NO.		1	
Client	NRG	BY	SJL	DATE	7/30/2021
Subject	Detention Basin Sizing	CHK'D.	RDS	DATE	7/30/2021

Problem Statement

Determine whether the detention basins are designed to convey the 25-year, 24-hour storm event without causing erosion or overtopping.

Given

- Elevation-area storage summaries for the Northeast and Southwest Detention Basins is • provided in Table A.7-1.
- All other key model parameters for the Northeast and Southwest Detention Basins is provided in Table A.7-2.
- The Northeast Detention Basin is equipped with two (2) forebays, the West Forebay and the East Forebay. The forebays collect stormwater from the Landfill prior to discharge into the Northeast Detention Basin.
- The Southwest Detention Basin is equipped with one (1) forebay, the Southwest Forebay. The forebay collects stormwater from the Landfill prior to discharge into the Southwest Detention Basin.
- All detention basins have been modeled with two (2) discharge methods. Discharge pipes and spillway systems that are designed to manage the flow out of each detention basin have been modeled. Additionally, exfiltration through native soils has also been modeled as a discharge method. According to the Sussex County Soil Survey, the rate of infiltration for the native soils is 5.95 in/hr. In accordance with the Sussex Conservation District guidelines, a rate of one-half the posted rate was used for design purposes; therefore, the modeled rate was 2.97 in/hr for the detention basins. The forebay systems are not modeled with this exfiltration parameter.

Calculations

HydroCAD was used to model the peak storage volume of both basins. The storage volume considers both the inflow, elevation-storage relationships of the basin, and outflow from the basin discharges including exfiltration.

Storage volumes were determined by measuring the area of each elevation contour in AutoCAD Civil 3D 2020 and entering the results into HydroCAD. HydroCAD then calculates a prismatic volume such that it can recalculate a storage volume at any bounded elevation. The normal water elevation for the forebay systems was set such that any dead-storage beneath this elevation was not accounted for as storage capacity.

The model results and sizing analysis for the basins including the peak elevation and discharge rates is provided in Table A.7-3.

Results

Based on the peak elevation results shown in Table A.7-3, the basins provide sufficient volume to detain the 25-year, 24-hour storm event without overtopping or causing downstream erosion. Please refer to Appendix A.8 for copies of the HydroCAD output files that supplement these conclusions.

	TAB Stormwater Basin Ele	LE A.7-1 evation-Storage Summary	
Elevation (ft MSL)	Surface Area (₶⁻)	Incremental Storage (π ⁻)	Cumulative Storage (Tt [~])
	Northeast	Basin System	
	West	t Forebay	
5.00	13,918	0	0
6.00	18,744	16,331	16,331
7.00	20,143	19,444	35,775
8.00	21,730	20,937	56,711
9.00	23,191	22,461	79,172
10.00	24,846	24,019	103,190
	East	Forebay	
5.00	16,828	0	0
6.00	19,651	18,240	18,240
7.00	21,422	20,537	38,776
8.00	23,109	22,266	61,042
9.00	24,775	23,942	84,984
10.00	28,341	26,558	111,542
	North	east Basin	·
5.00	241,617	0	0
6.00	266,595	254,106	254,106
7.00	275,843	271,219	525,325
8.00	286,099	280,971	806,296
9.00	296,412	291,256	1,097,552
10.00	306,793	301,603	1,399,154
	Southwest	Basin System	· ·
	Southw	est Forebay	
15.00	13,915	0	0
16.00	15,403	14.659	14,659
17.00	16.959	16.181	30.840
18.00	18,585	17,772	48,612
19.00	20,280	19,433	68,045
20.00	22.044	21.162	89.207
	South	west Basin	, -
15.00	129.810	0	0
16.00	134.325	132.068	132.068
17.00	138.901	136.613	268.681
18.00	143.540	141.221	409.901
19.00	148,240	145.890	555.791
20.00	153,002	150 621	706 412
20.00	103,002	100,021	100,412





	TABLE A.7-2 Basin / Forebay Outlet Structure Summary Table							
	Design Parameter Basin / Forebay Element Unit of Measure							
	West Forebay							
way	Weir Elevation	7.0	ft MSL					
r Spill	Weir Depth	3.0	ft					
Wei	Weir Bottom Width	36.0	ft					
	Weir Sideslopes	3	H:1V					
		East Forebay						
way	Weir Elevation	7.0	ft MSL					
r Spill	Weir Depth	3.0	ft					
Wei	Weir Bottom Width	20.0	ft					
	Weir Sideslopes	3	H:1V					
er		Northeast Basin						
kimme	Skimmer Elevation	5.0	ft MSL					
s	Approximate Full Flow	0.113	cubic feet per second					
ration	Exfiltration conductivity	2.970	inches per hour					
Exfilt	Groundwater Elevation	4.0	ft MSL					
		Southwest Forebay						
ture	Weir Elevation	17.0	ft MSL					
- Struc	Weir Depth	2.0	ft					
Weir	Weir Bottom Width	36.0	ft					
	Weir Sideslopes	2	H:1V					
		Southwest Basin						
arre	Weir Elevation	18.0	ft MSL					
Struct	Weir Depth	2.0	ft					
Weir	Weir Bottom Width	10.0	ft					
	Weir Sideslopes	3	H:1V					
ration	Exfiltration conductivity	2.970	inches per hour					
Exfilt	Groundwater Elevation	14.0	ft MSL					



Indian River Landfill

TABLE A.7-3 Basin Design Summary									
Basin / Forebay	Normal Water Level (ft MSL)	Top of Berm Elevation (ft MSL)	25-Year, 24-Hour Elevation (ft MSL)	Minimum Freeboard (ft)					
West Forebay	7.00	10.00	7.22	2.78					
East Forebay	7.00	10.00	7.22	2.78					
Northeast Basin	5.00	10.00	5.06	4.94					
Southwest Forebay	17.00	20.00	17.13	2.87					
Southwest Basin	15.00	20.00	15.03	4.97					





Appendix A.8 HydroCAD Output Files HydroCAD Output Files 25-Year, 24-Hour Storm Event



Summary for Subcatchment SC-A1: SC-A1

Runoff = 0.45 cfs @ 12.04 hrs, Volume= 0.041 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area	(ac) C	N Des	cription			
0.	744 3	39 >759	% Grass co	over, Good	HSG A	_
0.	744	100.	00% Pervi	ous Area		_
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
7.6	100	0.0350	0.22		Sheet Flow,	_
0.8	63	0.0350	1.31		Grass: Short n= 0.150 P2= 3.40" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps	
84	163	Total				

Subcatchment SC-A1: SC-A1



Summary for Subcatchment SC-A10: SC-A10

Runoff = 0.70 cfs @ 11.95 hrs, Volume= 0.033 af, Depth= 1.09"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"



Summary for Subcatchment SC-A11: SC-A11

Runoff = 0.65 cfs @ 11.95 hrs, Volume= 0.040 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

A	rea (sf)	CN I	Description								
	31,485	39 :	39 >75% Grass cover, Good, HSG A								
	31,485		100.00% Pe	ervious Are	а						
Тс	Length	Slope	Velocity	Capacity	Description						
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
1.7	47	0.3333	0.46		Sheet Flow,						
					Grass: Short	n= 0.150	P2= 3.40"				

Subcatchment SC-A11: SC-A11



Summary for Subcatchment SC-A12: SC-A12

Runoff = 0.51 cfs @ 11.93 hrs, Volume= 0.022 af, Depth= 1.58"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"



Summary for Subcatchment SC-A2: SC-A2

Runoff = 0.23 cfs @ 12.05 hrs, Volume= 0.021 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area	(ac) C	N Desc	cription		
0.	387 3	9 > 759	% Grass co	over, Good,	HSG A
0.	387	100.	00% Pervi	ous Area	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.6	100	0.0350	0.22		Sheet Flow,
1.1	84	0.0350	1.31		Grass: Short n= 0.150 P2= 3.40" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
8.7	184	Total			

Subcatchment SC-A2: SC-A2



. ..

Summary for Subcatchment SC-A3: SC-A3

Runoff = 0.63 cfs @ 11.98 hrs, Volume= 0.044 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

	Area ((ac) (CN	Desc	ription					
	0.8	809	39	>75%	6 Grass co	over, Good,	HSG A			
0.809 100.00% Pervious Area										
	Tc (min)	Length (feet)	S	lope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
	3.7	94	0.1	1809	0.42		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-A3: SC-A3



Summary for Subcatchment SC-A4: SC-A4

Runoff = 0.71 cfs @ 11.96 hrs, Volume= 0.044 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

	Area	(ac) (CN	Desc	ription					
	0.	801	39	>75%	6 Grass co	over, Good,	HSG A			
0.801 100.00% Pervious Area						ous Area				
	Tc (min)	Length (feet)	S	lope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
	2.1	62	0.3	3333	0.49	, , , , , , , , , , , , , , , , , , ,	Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-A4: SC-A4



Summary for Subcatchment SC-A5: SC-A5

Runoff = 0.38 cfs @ 11.95 hrs, Volume= 0.023 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area	(ac) C	N De	scription					
0.	429 🕄	39 >7	5% Grass c	over, Good,	, HSG A			
0.	429	10	0.00% Perv	ious Area				
Tc (min)	Length (feet)	Slop (ft/f	e Velocity) (ft/sec)	Capacity (cfs)	Description			
1.9	54	0.333	3 0.48		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-A5: SC-A5



Summary for Subcatchment SC-A6: SC-A6

Runoff = 3.14 cfs @ 11.90 hrs, Volume= 0.129 af, Depth= 3.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"



Summary for Subcatchment SC-A7: SC-A7

Runoff = 0.24 cfs @ 11.96 hrs, Volume= 0.015 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area (ac) C	N Des	cription						
0.2	270 3	39 >75°	% Grass co	over, Good,	HSG A				
0.270 100.00% Pervious Area									
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
2.4	72	0.3333	0.51		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"		

Subcatchment SC-A7: SC-A7


Summary for Subcatchment SC-A8: SC-A8

Runoff = 0.45 cfs @ 11.95 hrs, Volume= 0.027 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

A	rea (sf)	CN	Description									
	21,613	39	39 >75% Grass cover, Good, HSG A									
	21,613		100.00% Pe	ervious Are	а							
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description							
1.7	48	0.3333	0.47		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"					

Subcatchment SC-A8: SC-A8



. ..

Summary for Subcatchment SC-A9: SC-A9

Runoff = 0.61 cfs @ 11.95 hrs, Volume= 0.037 af, Depth= 0.66"

Area	(ac) C	IN Des	cription								
0.	676	39 >75°	% Grass co	over, Good,	HSG A						
0.	676										
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description						
1.8	50	0.3333	0.47		Sheet Flow, Grass: Short n= 0.1	150 P2=	3.40"				
Subcatchment SC-A9: SC-A9											



Summary for Subcatchment SC-B1: SC-B1

Runoff = 0.26 cfs @ 12.05 hrs, Volume= 0.023 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area	(ac) C	N Dese	cription			
0.	429 3	39 >759	% Grass co	over, Good,	, HSG A	
0.	429	100.	00% Pervi	ous Area		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
7.6	100	0.0350	0.22		Sheet Flow,	
1.1	83	0.0350	1.31		Grass: Short n= 0.150 P2= 3.40" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps	
87	183	Total				

Subcatchment SC-B1: SC-B1



Summary for Subcatchment SC-B10: SC-B10

Runoff = 0.51 cfs @ 11.95 hrs, Volume= 0.029 af, Depth= 0.72"



Summary for Subcatchment SC-B11: SC-B11

Runoff = 0.06 cfs @ 11.96 hrs, Volume= 0.003 af, Depth= 1.66"



Summary for Subcatchment SC-B12: SC-B12

Runoff = 0.89 cfs @ 11.96 hrs, Volume= 0.055 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

	Area	(ac) C	CN E	Desc	ription					
0.996 39 >75% Grass cover, Good, HSG A										
	0.	996	1	00.0	00% Pervi	ous Area				
	Tc (min)	Length (feet)	Slo (ft	pe /ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
	2.1	60	0.33	33	0.49		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-B12: SC-B12



Summary for Subcatchment SC-B13: SC-B13

Runoff = 0.24 cfs @ 11.96 hrs, Volume= 0.015 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area (ac) C	N Des	cription					
0.2	275 3	39 >75	% Grass co	over, Good,	HSG A			
0.2	275	100	.00% Pervi	ous Area				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
2.1	61	0.3333	0.49		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-B13: SC-B13



Λ

2 4 6 8

10 12 14 16 18

20

22 24 26

Time (hours)

28

30 32 34 36 38 40 42 44 46 48

Summary for Subcatchment SC-B14: SC-B14

Runoff = 0.99 cfs @ 11.95 hrs, Volume= 0.061 af, Depth= 0.66"



Summary for Subcatchment SC-B15: SC-B15

Runoff 0.52 cfs @ 11.95 hrs, Volume= 0.032 af, Depth= 0.66" =

Area (ac)	CN Des	cription									
0.585	39 >75	% Grass co	over, Good,	, HSG A							
0.585	100	.00% Pervi	ous Area								
Tc Lengt (min) (fee	h Slope t) (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description							
2.0 5	7 0.3333	0.48		Sheet Flow, Grass: Short n= 0.150 P2= 3.40"							
Subcatchment SC-B15: SC-B15											
Hydrograph											



Summary for Subcatchment SC-B16: SC-B16

Runoff = 0.29 cfs @ 11.94 hrs, Volume= 0.018 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area ((ac) C	CN D	esci	ription					
0.3	325	39 >	75%	Grass co	over, Good,	HSG A			
0.3	325	1	00.0	0% Pervi	ous Area				
Tc (min)	Length	Sloj (ft/	ce ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
1.0	24	0.33	33	0.41	(010)	Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	_

Subcatchment SC-B16: SC-B16



Summary for Subcatchment SC-B17: SC-B17

Runoff = 0.49 cfs @ 11.91 hrs, Volume= 0.021 af, Depth= 1.58"



Summary for Subcatchment SC-B2: SC-B2

Runoff = 0.54 cfs @ 12.05 hrs, Volume= 0.049 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

A	rea (sf)	CN D	escription						
38,907 39 >75% Grass cover, Good, HSG A									
	38,907	1	00.00% Pe	ervious Are	a				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
7.6	100	0.0350	0.22		Sheet Flow,				
1.0	75	0.0350	1.31		Grass: Short n= 0.150 P2= 3.40" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps				
8.6	175	Total							

Subcatchment SC-B2: SC-B2



Summary for Subcatchment SC-B3: SC-B3

Runoff = 1.10 cfs @ 12.22 hrs, Volume= 0.173 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

 Area	(ac) C	N Dese	cription			
3.	160 3	39 > 759	% Grass co	over, Good,	HSG A	
3.	160	100.	00% Pervi	ous Area		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
16.5	100	0.0050	0.10		Sheet Flow,	
 4.3	325	0.0323	1.26		Grass: Short n= 0.150 P2= 3.40" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps	
20.8	425	Total				

Subcatchment SC-B3: SC-B3



Summary for Subcatchment SC-B4: SC-B4

Runoff = 0.40 cfs @ 12.24 hrs, Volume= 0.065 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area	(ac) C	N Des	cription			
1.	183 3	39 >759	% Grass co	over, Good	HSG A	
1.	183	100.	00% Pervi	ous Area		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
16.5	100	0.0050	0.10		Sheet Flow,	
5.6	365	0.0237	1.08		Grass: Short n= 0.150 P2= 3.40" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps	
22.1	165	Total				

Subcatchment SC-B4: SC-B4



Summary for Subcatchment SC-B5: SC-B5

Runoff = 0.93 cfs @ 12.00 hrs, Volume= 0.070 af, Depth= 0.66"

Area ((ac) C	N Dese	cription		
1.2	270 3	9 > 759	% Grass co	over, Good,	HSG A
1.2	270	100.	00% Pervi	ous Area	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.5	100	0.0777	0.30		Sheet Flow,
0.0	10	0.3333	4.04		Grass: Short n= 0.150 P2= 3.40" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
5.5	110	Total			





Summary for Subcatchment SC-B6: SC-B6

Runoff = 0.03 cfs @ 11.95 hrs, Volume= 0.002 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area	(ac) C	N D	escription						
0.	031 🗧	39 >	75% Gras	s cover,	, Good,	HSG A			
0.	031	1(0.00% P	ervious	Area				
Tc (min)	Length (feet)	Slop (ft/	e Veloc ft) (ft/se	city Ca	pacity (cfs)	Description			
1.6	43	0.333	3 0.	46		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-B6: SC-B6



Summary for Subcatchment SC-B7: SC-B7

Runoff = 0.41 cfs @ 11.95 hrs, Volume= 0.025 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area	(ac) C	N Des	scription					
0.	460 🕄	39 >75	5% Grass c	over, Good,	, HSG A			
0.	460	100	.00% Pervi	ous Area				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
1.8	50	0.3333	0.47		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-B7: SC-B7



Summary for Subcatchment SC-B8: SC-B8

Runoff = 3.64 cfs @ 11.90 hrs, Volume= 0.149 af, Depth= 3.35"



Summary for Subcatchment SC-B9: SC-B9

Runoff = 0.11 cfs @ 11.96 hrs, Volume= 0.007 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

	A	rea (sf)	CN	Description					
		5,574	39	>75% Gras	s cover, Go	od, HSG A			
		5,574		100.00% Pe	ervious Are	а			
(1	Tc min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
	2.1	63	0.3333	0.49		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-B9: SC-B9



Summary for Subcatchment SC-C1: SC-C1

Runoff = 0.34 cfs @ 11.97 hrs, Volume= 0.023 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area	(ac) C	CN De	escription					
0.	424	39 >7	'5% Grass	cover, Good	, HSG A			
0.	424	10	0.00% Per	vious Area				
Tc (min)	Length (feet)	Slop (ft/f	e Velocity t) (ft/sec)	/ Capacity) (cfs)	Description			
3.3	62	0.107	8 0.31	l	Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-C1: SC-C1



Summary for Subcatchment SC-C10: SC-C10

Runoff = 0.16 cfs @ 11.91 hrs, Volume= 0.007 af, Depth= 1.58"



Summary for Subcatchment SC-C2: SC-C2

Runoff = 0.03 cfs @ 11.94 hrs, Volume= 0.002 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

A	rea (sf)	CN [Description					
	1,603	39 >	>75% Gras	s cover, Go	od, HSG A			
	1,603	1	100.00% Pe	ervious Are	а			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
0.7	17	0.3333	0.38	, , , , , , , , , , , , , , , , , , ,	Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-C2: SC-C2



Summary for Subcatchment SC-C3: SC-C3

Runoff = 0.49 cfs @ 11.95 hrs, Volume= 0.030 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Ar	ea (sf)	CN	Description					
2	24,010	39	>75% Gras	s cover, Go	ood, HSG A			
2	24,010		100.00% P	ervious Are	а			
Tc (min)	Length (feet)	Slope (ft/ft	velocity (ft/sec)	Capacity (cfs)	Description			
2.0	59	0.3333	0.49		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-C3: SC-C3



Summary for Subcatchment SC-C4: SC-C4

Runoff = 0.28 cfs @ 11.95 hrs, Volume= 0.017 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area	(ac) (CN I	Desc	ription					
0.	311	39 :	>75%	6 Grass co	over, Good,	HSG A			
0.	311		100.0	0% Pervi	ous Area				
Tc (min)	Length (feet)	Slo (f	ope t/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
2.0	59	0.33	333	0.49	· ·	Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-C4: SC-C4



Summary for Subcatchment SC-C5: SC-C5

Runoff = 0.47 cfs @ 11.95 hrs, Volume= 0.029 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area (ac) C	N Des	cription					
0.5	521 3	39 >75	% Grass c	over, Good,	HSG A			
0.5	521	100	.00% Pervi	ous Area				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
2.0	59	0.3333	0.49		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-C5: SC-C5



Summary for Subcatchment SC-C6: SC-C6

Runoff = 0.28 cfs @ 11.95 hrs, Volume= 0.017 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

A	rea (sf)	CN I	Description					
	13,643	39 >	>75% Gras	s cover, Go	od, HSG A			
	13,643		100.00% Pe	ervious Are	а			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
2.0	59	0.3333	0.49		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-C6: SC-C6



Summary for Subcatchment SC-C7: SC-C7

Runoff = 0.43 cfs @ 11.95 hrs, Volume= 0.026 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area	(ac) (CN Des	scription					
0	.481	39 >75	5% Grass c	over, Good,	, HSG A			
0	.481	100	.00% Pervi	ous Area				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
2.0	58	0.3333	0.48		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-C7: SC-C7



. ..

Summary for Subcatchment SC-C8: SC-C8

Runoff = 0.27 cfs @ 11.95 hrs, Volume= 0.016 af, Depth= 0.66"

Area	(ac)	CN	Desc	cription			
0.	300	39	>75%	6 Grass co	over, Good,	HSG A	
0.	300		100.	00% Pervi	ous Area		
Tc (min)	Lengt (fee	t)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
2.0	5	8 0	.3333	0.48		Sheet Flow, Grass: Short n= 0.150 P2= 3.40"	
				S	ubcatchn	nent SC-C8: SC-C8	
					Hydrog	graph	
	~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~						



Summary for Subcatchment SC-C9: SC-C9

Runoff = 0.06 cfs @ 11.94 hrs, Volume= 0.003 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area	(ac) C	CN E	Desc	ription					
0.	063	39 >	>75%	6 Grass co	over, Good,	HSG A			
0.	063	1	00.0	00% Pervi	ous Area				
Tc (min)	Length (feet)	Slo (ft	pe /ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
0.6	12	0.33	33	0.35	()	Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-C9: SC-C9



Summary for Subcatchment SC-D1: SC-D1

Runoff = 0.64 cfs @ 12.21 hrs, Volume= 0.097 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area	(ac) C	N Dese	cription			
1.	779 3	39 >759	% Grass co	over, Good	, HSG A	
1.	779	100.	00% Pervi	ous Area		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
16.5	100	0.0050	0.10		Sheet Flow,	
3.5	256	0.0301	1.21		Grass: Short n= 0.150 P2= 3.40" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps	
20.0	356	Total				

Subcatchment SC-D1: SC-D1



Summary for Subcatchment SC-D2: SC-D2

Runoff = 0.06 cfs @ 12.06 hrs, Volume= 0.006 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

A	rea (sf)	CN D	Description						
	4,530	39 >75% Grass cover, Good, HSG A							
	4,530	1	00.00% Pe	ervious Are	а				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
7.6	100	0.0350	0.22		Sheet Flow,				
1.8	143	0.0350	1.31		Grass: Short n= 0.150 P2= 3.40" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps				
9.4	2/13	Total							

Subcatchment SC-D2: SC-D2



Summary for Subcatchment SC-D3: SC-D3

Runoff = 0.67 cfs @ 12.05 hrs, Volume= 0.062 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

_	A	rea (sf)	CN D	escription				
		49,172	39 >75% Grass cover, Good, HSG A					
	49,172		100.00% Pervious Area			a		
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
	7.6	100	0.0350	0.22		Sheet Flow,		
	1.2	193	0.1352	2.57		Grass: Short n= 0.150 P2= 3.40" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps		
	8.8	293	Total				-	

Subcatchment SC-D3: SC-D3



Summary for Subcatchment SC-D4: SC-D4

Runoff = 0.90 cfs @ 11.96 hrs, Volume= 0.056 af, Depth= 0.66"



Summary for Subcatchment SC-D5: SC-D5

Runoff = 0.94 cfs @ 11.95 hrs, Volume= 0.057 af, Depth= 0.66"



. ..

Summary for Subcatchment SC-D6: SC-D6

Runoff = 0.13 cfs @ 11.94 hrs, Volume= 0.008 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area	(ac) (JN Des	cription					
0.	142	39 >75	% Grass co	over, Good,	HSG A			
0.	142	100.	00% Pervi	ous Area				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
0.7	15	0.3333	0.37		Sheet Flow, Grass: Short r	า= 0.150	P2= 3.40"	
			_					

Subcatchment SC-D6: SC-D6



Summary for Subcatchment SC-D7: SC-D7

Runoff = 0.22 cfs @ 11.91 hrs, Volume= 0.009 af, Depth= 1.49"


Summary for Reach LP-1A: LP-1A

 Inflow Area =
 1.131 ac, 0.00% Impervious, Inflow Depth = 0.66" for 25-yr,24-hr event

 Inflow =
 0.50 cfs @
 12.20 hrs, Volume=
 0.062 af

 Outflow =
 0.50 cfs @
 12.20 hrs, Volume=
 0.062 af, Atten= 0%, Lag= 0.3 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 8.90 fps, Min. Travel Time= 0.2 min Avg. Velocity = 4.46 fps, Avg. Travel Time= 0.3 min

Peak Storage= 5 cf @ 12.20 hrs Average Depth at Peak Storage= 0.10' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 110.50 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 88.1' Slope= 0.2386 '/' Inlet Invert= 90.02', Outlet Invert= 69.00'



Reach LP-1A: LP-1A



Summary for Reach LP-1B: LP-1B

 Inflow Area =
 1.940 ac, 0.00% Impervious, Inflow Depth = 0.66" for 25-yr,24-hr event

 Inflow =
 0.84 cfs @
 12.17 hrs, Volume=
 0.106 af

 Outflow =
 0.83 cfs @
 12.17 hrs, Volume=
 0.106 af, Atten= 1%, Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 9.98 fps, Min. Travel Time= 0.1 min Avg. Velocity = 4.91 fps, Avg. Travel Time= 0.3 min

Peak Storage= 7 cf @ 12.17 hrs Average Depth at Peak Storage= 0.13' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 104.94 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 88.3' Slope= 0.2152 '/' Inlet Invert= 69.00', Outlet Invert= 50.00'



Hydrograph Inflow Outflow 0.84 cfs 0.9 0.83 cfs Inflow Area=1.940 ac 0.85 0.8 Avg. Flow Depth=0.13' 0.75 Max Vel=9.98 fps 0.7 0.65 24.0" 0.6 **Round Pipe** 0.55 (cfs) 0.5 n=0.013 Flow 0.45 0.4 L=88.3' 0.35 S=0.2152 '/' 0.3 0.25 Capacity=104.94 cfs 0.2 0.15 0.1 0.05 0 Ż 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 0 Time (hours)

Reach LP-1B: LP-1B

Summary for Reach LP-1C: LP-1C

 Inflow Area =
 2.741 ac,
 0.00% Impervious, Inflow Depth =
 0.66" for 25-yr,24-hr event

 Inflow =
 1.19 cfs @
 12.16 hrs, Volume=
 0.150 af

 Outflow =
 1.18 cfs @
 12.16 hrs, Volume=
 0.150 af, Atten= 0%, Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 11.74 fps, Min. Travel Time= 0.1 min Avg. Velocity = 5.64 fps, Avg. Travel Time= 0.2 min

Peak Storage= 7 cf @ 12.16 hrs Average Depth at Peak Storage= 0.14' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 113.04 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 74.1' Slope= 0.2497 '/' Inlet Invert= 50.00', Outlet Invert= 31.50'





Reach LP-1C: LP-1C

Summary for Reach LP-1D: LP-1D

 Inflow Area =
 3.900 ac, 0.00% Impervious, Inflow Depth = 0.98" for 25-yr,24-hr event

 Inflow =
 3.11 cfs @
 12.01 hrs, Volume=
 0.317 af

 Outflow =
 3.10 cfs @
 12.01 hrs, Volume=
 0.317 af, Atten= 1%, Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 6.30 fps, Min. Travel Time= 0.1 min Avg. Velocity = 2.38 fps, Avg. Travel Time= 0.4 min

Peak Storage= 27 cf @ 12.01 hrs Average Depth at Peak Storage= 0.43' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 30.87 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 53.7' Slope= 0.0186 '/' Inlet Invert= 31.50', Outlet Invert= 30.50'



Hydrograph Inflow Outflow <u>3.11 cfs</u> 3.10 cfs Inflow Area=3.900 ac 3 Avg. Flow Depth=0.43' Max Vel=6.30 fps 24.0" **Round Pipe** Flow (cfs) 2 n=0.013 L=53.7' S=0.0186 '/' 1 Capacity=30.87 cfs 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Ò Ż 4 6 8 Time (hours)

Reach LP-1D: LP-1D

Summary for Reach LP-1E: LP-1E

 Inflow Area =
 4.396 ac, 0.00% Impervious, Inflow Depth = 0.94" for 25-yr,24-hr event

 Inflow =
 3.11 cfs @
 12.01 hrs, Volume=
 0.344 af

 Outflow =
 3.10 cfs @
 12.01 hrs, Volume=
 0.344 af, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 13.48 fps, Min. Travel Time= 0.1 min Avg. Velocity = 5.39 fps, Avg. Travel Time= 0.2 min

Peak Storage= 16 cf @ 12.01 hrs Average Depth at Peak Storage= 0.25' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 91.30 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 70.6' Slope= 0.1629 '/' Inlet Invert= 30.50', Outlet Invert= 19.00'





Reach LP-1E: LP-1E

Summary for Reach LP-1F: LP-1F

 Inflow Area =
 5.438 ac, 0.00% Impervious, Inflow Depth = 0.91" for 25-yr,24-hr event

 Inflow =
 3.60 cfs @
 12.03 hrs, Volume=
 0.414 af

 Outflow =
 3.60 cfs @
 12.03 hrs, Volume=
 0.414 af, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 17.08 fps, Min. Travel Time= 0.0 min Avg. Velocity = 6.92 fps, Avg. Travel Time= 0.1 min

Peak Storage= 8 cf @ 12.03 hrs Average Depth at Peak Storage= 0.24' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 120.21 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 40.2' Slope= 0.2823 '/' Inlet Invert= 19.00', Outlet Invert= 7.65'





Reach LP-1F: LP-1F

Summary for Reach LP-2A: LP-2A

 Inflow Area =
 1.322 ac, 0.00% Impervious, Inflow Depth = 0.66" for 25-yr,24-hr event

 Inflow =
 0.56 cfs @
 12.21 hrs, Volume=
 0.072 af

 Outflow =
 0.56 cfs @
 12.22 hrs, Volume=
 0.072 af, Atten= 0%, Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 8.92 fps, Min. Travel Time= 0.1 min Avg. Velocity = 4.48 fps, Avg. Travel Time= 0.2 min

Peak Storage= 4 cf @ 12.22 hrs Average Depth at Peak Storage= 0.10' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 105.78 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 59.6' Slope= 0.2186 '/' Inlet Invert= 90.03', Outlet Invert= 77.00'



Hydrograph Inflow Outflow 0 56 cfs 06 0.56 cfs Inflow Area=1.322 ac 0.55 Avg. Flow Depth=0.10' 0.5 Max Vel=8.92 fps 0.45 24.0" 0.4 **Round Pipe** (cfs) 0.35 n=0.013 Flow 0.3 L=59.6' 0.25 S=0.2186 '/' 0.2 Capacity=105.78 cfs 0.15 0.1 0.05 0 Ż 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 0 Time (hours)

Reach LP-2A: LP-2A

Summary for Reach LP-2B: LP-2B

 Inflow Area =
 6.966 ac,
 0.00% Impervious,
 Inflow Depth =
 0.66" for 25-yr,24-hr event

 Inflow =
 1.53 cfs @
 12.62 hrs,
 Volume=
 0.382 af

 Outflow =
 1.53 cfs @
 12.62 hrs,
 Volume=
 0.382 af,

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 12.08 fps, Min. Travel Time= 0.1 min Avg. Velocity = 6.61 fps, Avg. Travel Time= 0.2 min

Peak Storage= 10 cf @ 12.62 hrs Average Depth at Peak Storage= 0.17' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 105.51 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 82.8' Slope= 0.2175 '/' Inlet Invert= 77.00', Outlet Invert= 58.99'



Hydrograph Inflow Outflow 1 53 cfs 1.53 cfs Inflow Area=6.966 ac Avg. Flow Depth=0.17' Max Vel=12.08 fps 24.0" **Round Pipe** Flow (cfs) n=0.013 L=82.8' S=0.2175 '/' Capacity=105.51 cfs Ò Ż 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Time (hours)

Reach LP-2B: LP-2B

Summary for Reach LP-2C: LP-2C

 Inflow Area =
 8.088 ac,
 0.00% Impervious,
 Inflow Depth =
 0.84"
 for
 25-yr,24-hr event

 Inflow =
 3.20 cfs @
 12.01 hrs,
 Volume=
 0.563 af

 Outflow =
 3.18 cfs @
 12.01 hrs,
 Volume=
 0.563 af,

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 8.11 fps, Min. Travel Time= 0.1 min Avg. Velocity = 3.44 fps, Avg. Travel Time= 0.3 min

Peak Storage= 21 cf @ 12.01 hrs Average Depth at Peak Storage= 0.37' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 43.84 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 53.0' Slope= 0.0375 '/' Inlet Invert= 58.99', Outlet Invert= 57.00'





Reach LP-2C: LP-2C

Summary for Reach LP-2D: LP-2D

 Inflow Area =
 8.589 ac,
 0.00% Impervious,
 Inflow Depth =
 0.83"
 for
 25-yr,24-hr event

 Inflow =
 3.28 cfs @
 12.02 hrs,
 Volume=
 0.595 af
 0.595 af,
 Atten=
 1%,
 Lag=
 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 14.08 fps, Min. Travel Time= 0.1 min Avg. Velocity = 6.29 fps, Avg. Travel Time= 0.3 min

Peak Storage= 23 cf @ 12.02 hrs Average Depth at Peak Storage= 0.25' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 95.49 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 101.2' Slope= 0.1782 '/' Inlet Invert= 57.00', Outlet Invert= 38.97'





Reach LP-2D: LP-2D

Summary for Reach LP-2E: LP-2E

 Inflow Area =
 9.860 ac,
 0.00% Impervious,
 Inflow Depth =
 0.81"
 for
 25-yr,24-hr event

 Inflow =
 3.44 cfs @
 12.04 hrs,
 Volume=
 0.665 af
 0.665 af,
 Atten= 0%,
 Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 15.15 fps, Min. Travel Time= 0.1 min Avg. Velocity = 6.82 fps, Avg. Travel Time= 0.2 min

Peak Storage= 20 cf @ 12.04 hrs Average Depth at Peak Storage= 0.25' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 102.99 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 86.7' Slope= 0.2073 '/' Inlet Invert= 38.97', Outlet Invert= 21.00'



Reach LP-2E: LP-2E Hydrograph Inflow Outflow 3 44 cfs 3.43 cfs Inflow Area=9.860 ac Avg. Flow Depth=0.25' 3 Max Vel=15.15 fps 24.0" **Round Pipe** Flow (cfs) 2 n=0.013 L=86.7' S=0.2073 '/' 1 Capacity=102.99 cfs Ò Ż 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Time (hours)

Summary for Reach LP-2F: LP-2F

 Inflow Area =
 11.550 ac, 0.00% Impervious, Inflow Depth = 0.79" for 25-yr,24-hr event

 Inflow =
 3.75 cfs @
 12.07 hrs, Volume=
 0.757 af

 Outflow =
 3.75 cfs @
 12.07 hrs, Volume=
 0.757 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 13.60 fps, Min. Travel Time= 0.0 min Avg. Velocity = 6.13 fps, Avg. Travel Time= 0.1 min

Peak Storage= 7 cf @ 12.07 hrs Average Depth at Peak Storage= 0.29' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 85.44 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 25.1' Slope= 0.1426 '/' Inlet Invert= 21.00', Outlet Invert= 17.42'



Hydrograph Inflow Outflow 3 75 cfs 3.75 cfs 4 Inflow Area=11.550 ac Avg. Flow Depth=0.29' Max Vel=13.60 fps 3 24.0" **Round Pipe** Flow (cfs) n=0.013 2 L=25.1' S=0.1426 '/' Capacity=85.44 cfs 1 Ò Ż 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Time (hours)

Reach LP-2F: LP-2F

Summary for Reach LP-3A: LP-3A

 Inflow Area =
 0.461 ac, 0.00% Impervious, Inflow Depth = 0.66" for 25-yr,24-hr event

 Inflow =
 0.20 cfs @
 12.16 hrs, Volume=
 0.025 af

 Outflow =
 0.20 cfs @
 12.16 hrs, Volume=
 0.025 af, Atten= 1%, Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 7.18 fps, Min. Travel Time= 0.2 min Avg. Velocity = 3.76 fps, Avg. Travel Time= 0.3 min

Peak Storage= 2 cf @ 12.16 hrs Average Depth at Peak Storage= 0.06' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 120.40 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 66.2' Slope= 0.2832 '/' Inlet Invert= 83.59', Outlet Invert= 64.84'



Reach LP-3A: LP-3A



Summary for Reach LP-3B: LP-3B

 Inflow Area =
 1.323 ac, 0.00% Impervious, Inflow Depth = 0.66" for 25-yr,24-hr event

 Inflow =
 0.61 cfs @
 12.10 hrs, Volume=
 0.072 af

 Outflow =
 0.61 cfs @
 12.10 hrs, Volume=
 0.072 af, Atten= 0%, Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 9.88 fps, Min. Travel Time= 0.1 min Avg. Velocity = 4.86 fps, Avg. Travel Time= 0.2 min

Peak Storage= 4 cf @ 12.10 hrs Average Depth at Peak Storage= 0.10' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 117.51 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 69.9' Slope= 0.2698 '/' Inlet Invert= 64.84', Outlet Invert= 45.98'



Hydrograph Inflow Outflow 0 61 cfs 0.65 0.61 cfs Inflow Area=1.323 ac 0.6 Avg. Flow Depth=0.10' 0 55 Max Vel=9.88 fps 0.5 24.0" 0.45 **Round Pipe** 0.4 [−]low (cfs) 0.35 n=0.013 0.3 L=69.9' 0.25 S=0.2698 '/' 0.2 Capacity=117.51 cfs 0.15 0.1 0.05 0 10 12 14 16 18 20 Ż 4 6 8 22 24 26 28 30 32 34 36 38 40 42 44 46 48 0 Time (hours)

Reach LP-3B: LP-3B

Summary for Reach LP-3C: LP-3C

 Inflow Area =
 2.157 ac, 0.00% Impervious, Inflow Depth = 0.66" for 25-yr,24-hr event

 Inflow =
 1.05 cfs @
 12.09 hrs, Volume=
 0.118 af

 Outflow =
 1.04 cfs @
 12.10 hrs, Volume=
 0.118 af, Atten= 0%, Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 11.25 fps, Min. Travel Time= 0.1 min Avg. Velocity = 5.30 fps, Avg. Travel Time= 0.2 min

Peak Storage= 6 cf @ 12.09 hrs Average Depth at Peak Storage= 0.14' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 112.32 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 69.2' Slope= 0.2465 '/' Inlet Invert= 45.98', Outlet Invert= 28.92'



Hydrograph Inflow Outflow 1 05 cfs 1.04 cfs Inflow Area=2.157 ac 1 Avg. Flow Depth=0.14' Max Vel=11.25 fps 24.0" **Round Pipe** Flow (cfs) n=0.013 L=69.2' S=0.2465 '/' Capacity=112.32 cfs 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Ò Ż 4 6 Time (hours)

Reach LP-3C: LP-3C

Summary for Reach LP-3D: LP-3D

 Inflow Area =
 2.938 ac,
 0.00% Impervious, Inflow Depth =
 0.66" for 25-yr,24-hr event

 Inflow =
 1.46 cfs @
 12.09 hrs, Volume=
 0.161 af

 Outflow =
 1.46 cfs @
 12.09 hrs, Volume=
 0.161 af, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 7.68 fps, Min. Travel Time= 0.0 min Avg. Velocity = 3.46 fps, Avg. Travel Time= 0.1 min

Peak Storage= 4 cf @ 12.09 hrs Average Depth at Peak Storage= 0.22' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 56.44 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 19.6' Slope= 0.0622 '/' Inlet Invert= 28.92', Outlet Invert= 27.70'



Hydrograph Inflow Outflow 1 46 cfs 1.46 cfs Inflow Area=2.938 ac Avg. Flow Depth=0.22' Max Vel=7.68 fps 24.0" **Round Pipe** Flow (cfs) n=0.013 L=19.6' S=0.0622 '/' Capacity=56.44 cfs 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Ò Ż 4 6 Time (hours)

Reach LP-3D: LP-3D

Summary for Reach LP-4A: LP-4A

 Inflow Area =
 1.883 ac, 0.00% Impervious, Inflow Depth = 0.66" for 25-yr,24-hr event

 Inflow =
 0.50 cfs @
 12.52 hrs, Volume=
 0.103 af

 Outflow =
 0.49 cfs @
 12.53 hrs, Volume=
 0.103 af, Atten= 0%, Lag= 0.3 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 8.92 fps, Min. Travel Time= 0.2 min Avg. Velocity = 4.97 fps, Avg. Travel Time= 0.3 min

Peak Storage= 5 cf @ 12.52 hrs Average Depth at Peak Storage= 0.10' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 111.38 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 92.9' Slope= 0.2424 '/' Inlet Invert= 91.52', Outlet Invert= 69.00'



Hydrograph Inflow Outflow 0 50 cfs 0.55 0.49 cfs Inflow Area=1.883 ac 0.5 Avg. Flow Depth=0.10' 0.45 Max Vel=8.92 fps 0.4 24.0" 0.35 **Round Pipe** (cfs) 0.3 n=0.013 Nol= 0.25 L=92.9' 02 S=0.2424 '/' 0.15 Capacity=111.38 cfs 0.1 0.05 0 Ò Ż 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Time (hours)

Reach LP-4A: LP-4A

Summary for Reach LP-4B: LP-4B

 Inflow Area =
 3.012 ac,
 0.00% Impervious, Inflow Depth =
 0.66" for 25-yr,24-hr event

 Inflow =
 0.74 cfs @
 12.48 hrs, Volume=
 0.165 af

 Outflow =
 0.74 cfs @
 12.48 hrs, Volume=
 0.165 af, Atten= 0%, Lag= 0.3 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 10.48 fps, Min. Travel Time= 0.1 min Avg. Velocity = 5.81 fps, Avg. Travel Time= 0.2 min

Peak Storage= 5 cf @ 12.48 hrs Average Depth at Peak Storage= 0.11' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 117.77 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 73.8' Slope= 0.2710 '/' Inlet Invert= 69.00', Outlet Invert= 49.00'



Hydrograph Inflow Outflow 0 74 cfs 0.8 0.74 cfs Inflow Area=3.012 ac 0.75 0.7 Avg. Flow Depth=0.11' 0.65 Max Vel=10.48 fps 0.6 24.0" 0.55 0.5 **Round Pipe** (cfs) 0.45 n=0.013 Flow 0.4 0.35 L=73.8' 0.3 S=0.2710 '/' 0.25 Capacity=117.77 cfs 0.2 0.15 0.1 0.05 0 Ż 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 0 Time (hours)

Reach LP-4B: LP-4B

Summary for Reach LP-4C: LP-4C

 Inflow Area =
 4.026 ac, 0.00% Impervious, Inflow Depth = 0.66" for 25-yr,24-hr event

 Inflow =
 0.93 cfs @
 12.46 hrs, Volume=
 0.221 af

 Outflow =
 0.93 cfs @
 12.46 hrs, Volume=
 0.221 af, Atten= 0%, Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 10.88 fps, Min. Travel Time= 0.1 min Avg. Velocity = 6.06 fps, Avg. Travel Time= 0.2 min

Peak Storage= 6 cf @ 12.46 hrs Average Depth at Peak Storage= 0.13' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 112.32 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 69.0' Slope= 0.2465 '/' Inlet Invert= 49.00', Outlet Invert= 31.99'



Hydrograph Inflow Outflow 0.93 cfs 0.93 cfs Inflow Area=4.026 ac Avg. Flow Depth=0.13' Max Vel=10.88 fps 24.0" **Round Pipe** Flow (cfs) n=0.013 L=69.0' S=0.2465 '/' Capacity=112.32 cfs Ò Ż 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Time (hours)

Reach LP-4C: LP-4C

Summary for Reach LP-4D: LP-4D

 Inflow Area =
 5.075 ac,
 0.00% Impervious, Inflow Depth =
 0.66" for 25-yr,24-hr event

 Inflow =
 1.24 cfs @
 12.21 hrs, Volume=
 0.278 af

 Outflow =
 1.24 cfs @
 12.21 hrs, Volume=
 0.278 af, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 7.64 fps, Min. Travel Time= 0.1 min Avg. Velocity = 3.99 fps, Avg. Travel Time= 0.1 min

Peak Storage= 5 cf @ 12.21 hrs Average Depth at Peak Storage= 0.20' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 60.10 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 28.2' Slope= 0.0706 '/' Inlet Invert= 31.99', Outlet Invert= 30.00'



Hydrograph Inflow Outflow 1 24 cfs 1.24 cfs Inflow Area=5.075 ac Avg. Flow Depth=0.20' Max Vel=7.64 fps 1 24.0" **Round Pipe** Flow (cfs) n=0.013 L=28.2' S=0.0706 '/' Capacity=60.10 cfs Ò Ż 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Time (hours)

Reach LP-4D: LP-4D

Summary for Reach PC-1: PC-1

 Inflow Area =
 0.216 ac,
 0.00% Impervious,
 Inflow Depth =
 0.94"
 for 25-yr,24-hr event

 Inflow =
 0.33 cfs @
 11.92 hrs,
 Volume=
 0.017 af

 Outflow =
 0.22 cfs @
 12.07 hrs,
 Volume=
 0.017 af,
 Atten= 33%,
 Lag= 9.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 1.15 fps, Min. Travel Time= 6.2 min Avg. Velocity = 0.65 fps, Avg. Travel Time= 11.0 min

Peak Storage= 83 cf @ 11.97 hrs Average Depth at Peak Storage= 0.02' Bank-Full Depth= 1.00' Flow Area= 9.0 sf, Capacity= 109.81 cfs

8.00' x 1.00' deep channel, n= 0.013 Concrete, trowel finish Side Slope Z-value= 1.0 '/' Top Width= 10.00' Length= 428.0' Slope= 0.0146 '/' Inlet Invert= 36.24', Outlet Invert= 30.00'



Reach PC-1: PC-1



Summary for Reach PC-2: PC-2

 Inflow Area =
 8.345 ac, 0.00% Impervious, Inflow Depth = 0.67" for 25-yr,24-hr event

 Inflow =
 2.50 cfs @
 12.13 hrs, Volume=
 0.466 af

 Outflow =
 2.45 cfs @
 12.20 hrs, Volume=
 0.466 af, Atten= 2%, Lag= 3.9 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 2.49 fps, Min. Travel Time= 1.9 min Avg. Velocity = 0.98 fps, Avg. Travel Time= 4.9 min

Peak Storage= 288 cf @ 12.16 hrs Average Depth at Peak Storage= 0.12' Bank-Full Depth= 1.00' Flow Area= 9.0 sf, Capacity= 81.37 cfs

8.00' x 1.00' deep channel, n= 0.013 Concrete, trowel finish Side Slope Z-value= 1.0 '/' Top Width= 10.00' Length= 287.3' Slope= 0.0080 '/' Inlet Invert= 30.00', Outlet Invert= 27.70'



Reach PC-2: PC-2



Summary for Reach PC-3: PC-3

 Inflow Area =
 20.380 ac, 0.00% Impervious, Inflow Depth = 0.74" for 25-yr,24-hr event

 Inflow =
 6.07 cfs @
 12.16 hrs, Volume=
 1.262 af

 Outflow =
 5.79 cfs @
 12.27 hrs, Volume=
 1.262 af, Atten= 5%, Lag= 6.6 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 3.85 fps, Min. Travel Time= 4.0 min Avg. Velocity = 1.37 fps, Avg. Travel Time= 11.1 min

Peak Storage= 1,388 cf @ 12.21 hrs Average Depth at Peak Storage= 0.19' Bank-Full Depth= 1.00' Flow Area= 9.0 sf, Capacity= 96.54 cfs

8.00' x 1.00' deep channel, n= 0.013 Concrete, trowel finish Side Slope Z-value= 1.0 '/' Top Width= 10.00' Length= 912.3' Slope= 0.0113 '/' Inlet Invert= 27.70', Outlet Invert= 17.42'



Reach PC-3: PC-3



Summary for Reach PC-4: PC-4

Inflow Area = 21.273 ac. 0.00% Impervious, Inflow Depth = 0.75" for 25-yr,24-hr event Inflow 5.98 cfs @ 12.27 hrs, Volume= 1.324 af = Outflow 5.74 cfs @ 12.39 hrs, Volume= = 1.324 af, Atten= 4%, Lag= 7.2 min Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 3.94 fps, Min. Travel Time= 4.1 min Avg. Velocity = 1.57 fps, Avg. Travel Time= 10.2 min Peak Storage= 1,422 cf @ 12.32 hrs Average Depth at Peak Storage= 0.22' Bank-Full Depth= 2.00' Flow Area= 22.0 sf, Capacity= 299.78 cfs 6.00' x 2.00' deep channel, n= 0.013 Concrete, trowel finish Side Slope Z-value= 2.0 3.0 '/' Top Width= 16.00' Length= 967.1' Slope= 0.0099 '/' Inlet Invert= 17.42', Outlet Invert= 7.83' ‡ Reach PC-4: PC-4 Hydrograph Inflow
Outflow 5.98 cfs Inflow Area=21.273 ac 5.74 cfs 6 Avg. Flow Depth=0.22' 5-Max Vel=3.94 fps n=0.013 ⁻low (cfs) L=967.1' S=0.0099 '/' 3-Capacity=299.78 cfs 2 1 0-4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 0 2

Time (hours)

Summary for Reach TB-A1: TB-A1



Summary for Reach TB-A2: TB-A2



Summary for Reach TB-A3: TB-A3



Summary for Reach TB-A4: TB-A4



Summary for Reach TB-A5: TB-A5



Summary for Reach TB-A6: TB-A6



Summary for Reach TB-A7: TB-A7



Summary for Reach TB-A8: TB-A8



Summary for Reach TB-A9: TB-A9



Summary for Reach TB-B1: TB-B1



Summary for Reach TB-B10: TB-B10



Summary for Reach TB-B11: TB-B11


Summary for Reach TB-B12: TB-B12



Summary for Reach TB-B13: TB-B13



Summary for Reach TB-B14: TB-B14



Summary for Reach TB-B2: TB-B2



Summary for Reach TB-B3: TB-B3



Summary for Reach TB-B4: TB-B4



Summary for Reach TB-B5: TB-B5



Summary for Reach TB-B6: TB-B6



Summary for Reach TB-B7: TB-B7



Summary for Reach TB-B8: TB-B8



Summary for Reach TB-B9: TB-B9



Summary for Reach TB-C1: TB-C1



Summary for Reach TB-C2: TB-C2



Summary for Reach TB-C3: TB-C3



Summary for Reach TB-C4: TB-C4



Summary for Reach TB-C5: TB-C5



Summary for Reach TB-C6: TB-C6



Summary for Reach TB-C7: TB-C7



Summary for Reach TB-C8: TB-C8



Summary for Reach TB-D1: TB-D1



Type II 24-hr 25-yr,24-hr Rainfall=6.68" Printed 7/30/2021 ions LLC Page 103

Summary for Reach TB-D2: TB-D2



Summary for Reach TB-D3: TB-D3



Summary for Reach TB-D4: TB-D4



Summary for Reach TB-D5: TB-D5



Summary for Link Link 1: Into NE Basin

Inflow A	Area :	=	26.711 ac,	0.00% Impervious,	Inflow Depth = 0	0.78" for 25-yr,24-hr event
Inflow	=	=	7.19 cfs @	12.37 hrs, Volume	= 1.739 a	f
Primary	/ =	=	7.19 cfs @	12.37 hrs, Volume	= 1.739 a	f, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs



Link Link 1: Into NE Basin

Summary for Subcatchment SC-E1: SC-E1

Runoff = 1.80 cfs @ 12.27 hrs, Volume= 0.314 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

 Area	(ac) C	N Dese	cription			
5.	738 3	39 > 759	% Grass co	over, Good,	HSG A	
5.	738	100.	00% Pervi	ous Area		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
11.1	100	0.0136	0.15		Sheet Flow,	-
 13.2	643	0.0134	0.81		Grass: Short n= 0.150 P2= 3.40" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps	
24.3	743	Total				





Summary for Subcatchment SC-E2: SC-E2

Runoff = 0.13 cfs @ 11.96 hrs, Volume= 0.008 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area (ac) C	N Dese	cription					
0.1	150 3	39 >75°	% Grass co	over, Good,	HSG A			
0.1	150	100.	00% Pervi	ous Area				
Tc (min)	Length	Slope	Velocity	Capacity	Description			
2.3	55	0.2210	0.41	(00)	Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-E2: SC-E2



Summary for Subcatchment SC-E3: SC-E3

Runoff = 0.28 cfs @ 11.96 hrs, Volume= 0.017 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

	Area	(ac)	CN	Desc	ription					
	0.	313	39	>75%	6 Grass co	over, Good,	HSG A			
0.313 100.00% Pervious Area										
	Tc (min)	Length (feet)	n S)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
	2.1	63	3 0.	3333	0.49	· · · · · ·	Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-E3: SC-E3



Summary for Subcatchment SC-E4: SC-E4

Runoff = 0.34 cfs @ 11.95 hrs, Volume= 0.021 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area	(ac) (CN	Desc	ription					
0.	385	39	>75%	6 Grass co	over, Good,	HSG A			
 0.	385		100.0	00% Pervi	ous Area				
Tc (min)	Length (feet)	SI (lope ′ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
1.9	56	0.3	333	0.48		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	-

Subcatchment SC-E4: SC-E4



Summary for Subcatchment SC-E5: SC-E5

Runoff = 0.40 cfs @ 11.96 hrs, Volume= 0.025 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area	(ac) C	CN	Desc	ription						
0.	462	39	>75%	6 Grass co	over, Good,	HSG A				
0.462 100.00% Pervious Area										
Tc (min)	Length (feet)	SI (lope ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
2.4	72	0.3	333	0.51		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"		

Subcatchment SC-E5: SC-E5



Summary for Subcatchment SC-E6: SC-E6

Runoff = 1.47 cfs @ 11.90 hrs, Volume= 0.060 af, Depth= 2.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"



Summary for Subcatchment SC-E7: SC-E7

Runoff = 0.20 cfs @ 11.95 hrs, Volume= 0.012 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area (ac) (CN Des	scription					
0.2	220	39 >75	% Grass c	over, Good,	, HSG A			
0.2	220	100	.00% Pervi	ous Area				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
1.7	47	0.3333	0.46		Sheet Flow, Grass: Short	n= 0 150	P2= 3 40"	

Subcatchment SC-E7: SC-E7



Summary for Subcatchment SC-E8: SC-E8

Runoff = 0.52 cfs @ 11.90 hrs, Volume= 0.021 af, Depth= 2.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"



Summary for Subcatchment SC-F1: SC-F1

Runoff = 0.83 cfs @ 11.96 hrs, Volume= 0.053 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area	(ac) (CN	Desc	ription					
0.	967	39	>75%	6 Grass co	over, Good,	HSG A			
0.	967		100.0	00% Pervi	ous Area				
Tc (min)	Length (feet)	S	lope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
2.7	71	0.2	2394	0.44		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-F1: SC-F1



Summary for Subcatchment SC-F2: SC-F2

Runoff = 1.08 cfs @ 11.96 hrs, Volume= 0.067 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"



Time (hours)

Summary for Subcatchment SC-F3: SC-F3

Runoff = 1.21 cfs @ 11.96 hrs, Volume= 0.074 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"


Summary for Subcatchment SC-F4: SC-F4

Runoff = 0.76 cfs @ 11.95 hrs, Volume= 0.046 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area	(ac) (CN	Desc	ription					
0.	845	39	>75%	6 Grass co	over, Good,	HSG A			
0.	845		100.0	00% Pervi	ous Area				
Tc (min)	Length (feet)	S	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
1.7	47	0.3	3333	0.46	, , , , , , , , , , , , , , , , , , ,	Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-F4: SC-F4



Summary for Subcatchment SC-F5: SC-F5

Runoff = 2.35 cfs @ 11.90 hrs, Volume= 0.096 af, Depth= 2.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"



Summary for Subcatchment SC-G1: SC-G1

Runoff = 0.34 cfs @ 12.05 hrs, Volume= 0.032 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area ((ac) C	N Dese	cription			
0.5	589 3	39 > 759	% Grass co	over, Good,	HSG A	
0.5	589	100.	00% Pervi	ous Area		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
8.4	100	0.0272	0.20		Sheet Flow,	
0.9	133	0.1372	2.59		Grass: Short n= 0.150 P2= 3.40" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps	
93	233	Total				

Subcatchment SC-G1: SC-G1



Summary for Subcatchment SC-G10: SC-G10

Runoff = 1.43 cfs @ 11.90 hrs, Volume= 0.058 af, Depth= 2.57"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"



Summary for Subcatchment SC-G2: SC-G2

Runoff = 0.64 cfs @ 12.06 hrs, Volume= 0.060 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area (a	ac) C	N Desc	cription			
1.1	03 3	9 >759	% Grass co	over, Good,	HSG A	
1.1	03	100.	00% Pervi	ous Area		
Tc I (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
7.0	100	0.0426	0.24		Sheet Flow,	
2.4	240	0.0568	1.67		Grass: Short n= 0.150 P2= 3.40" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps	
94	340	Total				

Subcatchment SC-G2: SC-G2



Summary for Subcatchment SC-G3: SC-G3

Runoff = 0.38 cfs @ 11.96 hrs, Volume= 0.024 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area	(ac) C	N De	scription					
0.	433 🕄	39 >7	5% Grass c	over, Good,	, HSG A			
0.	433	100	0.00% Pervi	ious Area				
Tc (min)	Length (feet)	Slope (ft/ft	e Velocity) (ft/sec)	Capacity (cfs)	Description			
2.1	63	0.3333	0.49		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-G3: SC-G3



Summary for Subcatchment SC-G4: SC-G4

Runoff = 0.36 cfs @ 11.96 hrs, Volume= 0.022 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area (ac) C	N De	scription					
0.4	403 3	39 >7	5% Grass c	over, Good	, HSG A			
0.4	403	10	0.00% Pervi	ious Area				
Tc (min)	Length (feet)	Slope (ft/ft	e Velocity) (ft/sec)	Capacity (cfs)	Description			
2.1	61	0.3333	3 0.49		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-G4: SC-G4



Summary for Subcatchment SC-G5: SC-G5

Runoff = 0.39 cfs @ 11.96 hrs, Volume= 0.024 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

A	rea (sf)	CN I	Description					
	19,331	39 :	>75% Gras	s cover, Go	od, HSG A			
	19,331		100.00% Pe	ervious Are	а			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
2.1	63	0.3333	0.49		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-G5: SC-G5



Summary for Subcatchment SC-G6: SC-G6

Runoff = 0.36 cfs @ 11.96 hrs, Volume= 0.022 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area	(ac)	CN	Desc	ription					
0.	404	39	>75%	6 Grass co	over, Good,	HSG A			
0.	404		100.0	00% Pervi	ous Area				
Tc (min)	Length (feet	n S	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
2.1	61	0.3	3333	0.49		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-G6: SC-G6



Summary for Subcatchment SC-G7: SC-G7

Runoff = 0.20 cfs @ 11.95 hrs, Volume= 0.012 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area	(ac) C	CN	Desc	ription					
0.	220	39	>75%	6 Grass co	over, Good,	HSG A			
0.	220		100.0	0% Pervi	ous Area				
Tc (min)	Length (feet)	SI (lope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
1.6	43	0.3	3333	0.46		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-G7: SC-G7



Summary for Subcatchment SC-G8: SC-G8

Runoff = 0.57 cfs @ 11.90 hrs, Volume= 0.023 af, Depth= 2.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"



Summary for Subcatchment SC-G9: SC-G9

Runoff = 0.72 cfs @ 11.96 hrs, Volume= 0.045 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area	(ac) (CN I	Desc	ription					
0.	814	39 >	>75%	6 Grass co	over, Good,	HSG A			
0.	814		100.0	00% Pervi	ous Area				
Tc (min)	Length (feet)	Slo (f	ope t/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
2.2	64	0.33	333	0.49		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-G9: SC-G9



. ..

Summary for Subcatchment SC-H1: SC-H1

Runoff = 0.48 cfs @ 11.96 hrs, Volume= 0.029 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

AI	rea (st)	CN I	Description					
	23,385	39 :	>75% Gras	s cover, Go	od, HSG A			
	23,385		100.00% Pe	ervious Area	а			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
2.1	60	0.3333	0.49		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-H1: SC-H1



Summary for Subcatchment SC-H10: SC-H10

Runoff = 2.71 cfs @ 11.93 hrs, Volume= 0.116 af, Depth= 3.15"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

 Area (ac)	CN	Desc	ription					
0.1	154	39	>75%	6 Grass co	over, Good,	HSG A			
 0.2	287	83	Pave	d roads w	/open ditch	es, 50% imp, ł	ISG A		
0.4	141	68	Weig	hted Aver	age				
0.2	297		67.46	5% Pervio	us Area				
0.1	143		32.54	4% Imperv	vious Area				
Tc (min)	Length (feet	ו 5)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
2.6	31	1 0.	0483	0.20		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-H10: SC-H10



Summary for Subcatchment SC-H2: SC-H2

Runoff = 0.64 cfs @ 11.97 hrs, Volume= 0.043 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area (a	ic) C	N Des	cription					
0.77	78 3	9 >75°	% Grass co	over, Good,	HSG A			
0.77	78	100.	00% Pervi	ous Area				
Tc L (min)	_ength (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
3.1	100	0.3333	0.54		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-H2: SC-H2



Summary for Subcatchment SC-H3: SC-H3

Runoff = 0.73 cfs @ 11.96 hrs, Volume= 0.045 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area ((ac) C	CN De	scription			
0.8	824 🗧	39 >7	5% Grass c	over, Good	, HSG A	
0.8	824	10	0.00% Perv	ious Area		
Tc (min)	Length (feet)	Slope (ft/ft	e Velocity) (ft/sec)	Capacity (cfs)	Description	
2.1	61	0.3333	3 0.49		Sheet Flow, . Grass: Short n= 0.150 P2= 3.40"	

Subcatchment SC-H3: SC-H3



Summary for Subcatchment SC-H4: SC-H4

Runoff = 0.64 cfs @ 11.95 hrs, Volume= 0.039 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area (<u>ac)</u> C	N Des	scription					
0.7	709 3	39 >75	5% Grass c	over, Good,	, HSG A			
0.7	709	100).00% Pervi	ous Area				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
1.8	51	0.3333	0.47		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-H4: SC-H4



Summary for Subcatchment SC-H5: SC-H5

Runoff = 0.87 cfs @ 11.96 hrs, Volume= 0.054 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area	(ac) (CN	Desc	ription					
0.	977	39	>75%	6 Grass co	over, Good,	HSG A			
0.	977		100.0	00% Pervi	ous Area				
Tc (min)	Length (feet)	SI (lope ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
2.1	61	0.3	333	0.49		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-H5: SC-H5



Summary for Subcatchment SC-H6: SC-H6

Runoff = 0.83 cfs @ 11.95 hrs, Volume= 0.051 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

A	rea (sf)	CN I	Description					
	40,490	39 :	>75% Gras	s cover, Go	od, HSG A			
	40,490		100.00% Pe	ervious Are	а			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
1.9	55	0.3333	0.48		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-H6: SC-H6



Summary for Subcatchment SC-H7: SC-H7

Runoff = 0.35 cfs @ 11.96 hrs, Volume= 0.022 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area (ac) C	N De	escription					
0.3	399 (39 >7	'5% Grass	cover, Good	, HSG A			
0.3	399	10	0.00% Per	vious Area				
Tc (min)	Length (feet)	Slop (ft/f	e Velocit t) (ft/sec	y Capacity	Description			
2.1	61	0.333	3 0.49	9	Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-H7: SC-H7



Summary for Subcatchment SC-H8: SC-H8

Runoff = 1.01 cfs @ 11.93 hrs, Volume= 0.043 af, Depth= 2.96"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

_	Area	(ac)	CN	Desc	ription		
	0.	055	39	>75%	6 Grass co	over, Good	, HSG A
	0.	082	76	Grav	el roads, H	ISG A	
*	0.	038	83	Pave	ed roads w	/open ditch	nes, 50% imp, HSG A
	0.	175	66	Weig	hted Aver	age	
	0.	156		89.14	4% Pervio	us Area	
	0.	019		10.86	6% Imperv	rious Area	
	Tc (min)	Lengt (feet	h S t)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	2.6	2	70.	0370	0.17		Sheet Flow,
							Grass: Short n= 0.150 P2= 3.40"





Summary for Subcatchment SC-H9: SC-H9

Runoff = 0.62 cfs @ 11.96 hrs, Volume= 0.039 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area (ac)) CI	N Des	cription					
0.706	63	9 >759	% Grass co	over, Good,	HSG A			
0.706	;	100.	00% Pervi	ous Area				
Tc Le (min) (ngth feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
2.2	66	0.3333	0.50		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-H9: SC-H9



Summary for Subcatchment SC-I1: SC-I1

Runoff = 0.84 cfs @ 12.12 hrs, Volume= 0.105 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area	(ac) C	N Dese	cription			
1.	908 3	39 >759	% Grass co	over, Good,	, HSG A	
1.	908	100.	00% Pervi	ous Area		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
7.0	100	0.0427	0.24		Sheet Flow,	
7.6	388	0.0147	0.85		Grass: Short n= 0.150 P2= 3.40" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps	
1/ 6	188	Total				

Subcatchment SC-I1: SC-I1



Summary for Subcatchment SC-I2: SC-I2

Runoff = 0.46 cfs @ 11.97 hrs, Volume= 0.030 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area (ac) C	N Des	cription					
0.5	543 3	39 >75	% Grass co	over, Good,	HSG A			
0.5	543	100.	.00% Pervi	ous Area				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
2.9	91	0.3333	0.53		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-I2: SC-I2



Summary for Subcatchment SC-I3: SC-I3

Runoff = 0.16 cfs @ 11.95 hrs, Volume= 0.010 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area (a	ac) C	N Des	cription					
0.1	78 3	9 >75°	% Grass co	over, Good,	, HSG A			
0.1	78	100.	00% Pervi	ous Area				
Tc I (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
1.8	50	0.3333	0.47	(010)	Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-I3: SC-I3



Summary for Subcatchment SC-I4: SC-I4

Runoff = 0.05 cfs @ 11.95 hrs, Volume= 0.003 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area	(ac) C	N Des	cription					
0.	059 3	39 >75	% Grass c	over, Good,	HSG A			
0.	059	100	.00% Pervi	ous Area				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
1.9	54	0.3333	0.48	()	Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-I4: SC-I4



Summary for Subcatchment SC-I5: SC-I5

Runoff = 0.35 cfs @ 11.96 hrs, Volume= 0.022 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area ((ac) C	N Des	cription					
0.4	402 3	39 >75	% Grass co	over, Good,	HSG A			
0.4	402	100	.00% Pervi	ous Area				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
2.4	74	0.3333	0.51		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-I5: SC-I5



Summary for Subcatchment SC-I6: SC-I6

Runoff = 0.86 cfs @ 11.90 hrs, Volume= 0.035 af, Depth= 2.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area (ac)	CN	Desc	cription				
0.071	39) >759	% Grass co	over, Good	, HSG A		
0.085 83 Paved roads w/open ditches, 50% imp, HSG A							
0.156	63	8 Weig	ghted Aver	age			
0.114		72.7	6% Pervio	us Area			
0.042		27.2	4% Imperv	vious Area			
Tc Le (min) (1	ngth feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
0.2	28	0.1698	2.59		Sheet Flow, Smooth surfaces n= 0.011 P2= 3.40"		

Subcatchment SC-I6: SC-I6



Summary for Subcatchment SC-J1: SC-J1

Runoff = 1.09 cfs @ 12.17 hrs, Volume= 0.154 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

 Area	(ac) C	N Dese	cription		
2.	809 3	9 > 759	% Grass co	over, Good,	HSG A
2.	809	100.	00% Pervi	ous Area	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
 9.0	100	0.0230	0.19		Sheet Flow,
 8.8	484	0.0172	0.92		Grass: Short n= 0.150 P2= 3.40" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
17.8	584	Total			

Subcatchment SC-J1: SC-J1



Summary for Subcatchment SC-J2: SC-J2

Runoff = 0.12 cfs @ 11.96 hrs, Volume= 0.008 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

A	rea (sf)	CN D	Description					
	6,251	39 >	75% Gras	s cover, Go	od, HSG A			
	6,251	1	00.00% Pe	ervious Are	а			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
2.5	78	0.3333	0.51		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-J2: SC-J2



Summary for Subcatchment SC-J3: SC-J3

Runoff = 0.26 cfs @ 11.96 hrs, Volume= 0.016 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"





Summary for Subcatchment SC-J4: SC-J4

Runoff = 0.35 cfs @ 11.96 hrs, Volume= 0.022 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area	(ac) (CN	Desc	ription					
0.	399	39	>75%	6 Grass co	over, Good,	HSG A			
0.	399		100.0	00% Pervi	ous Area				
Tc (min)	Length (feet)	S	lope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
2.1	63	0.3	3333	0.49		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-J4: SC-J4



Summary for Subcatchment SC-J5: SC-J5

Runoff = 0.37 cfs @ 11.96 hrs, Volume= 0.023 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

ŀ	Area	(ac)	CN	Desc	cription							
	0.	417	39	>75%	6 Grass co	over, Good,	HSG A					
	0.417 100.00% Pervious Area											
(r	Tc nin)	Length (feet	n S)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
	2.1	62	2 0.	.3333	0.49		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"			

Subcatchment SC-J5: SC-J5



Summary for Subcatchment SC-J6: SC-J6

Runoff = 1.43 cfs @ 11.90 hrs, Volume= 0.058 af, Depth= 2.86"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"



Summary for Subcatchment SC-K1: SC-K1

Runoff = 0.98 cfs @ 12.21 hrs, Volume= 0.153 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

	Area	(ac) C	N Desc	cription		
	2.	801 3	9 > 759	% Grass co	over, Good,	HSG A
	2.	801	100.	00% Pervi	ous Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	12.5	100	0.0100	0.13		Sheet Flow,
	8.1	472	0.0191	0.97		Grass: Short n= 0.150 P2= 3.40" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
	20.6	572	Total			

Subcatchment SC-K1: SC-K1



Summary for Subcatchment SC-K10: SC-K10

Runoff = 0.84 cfs @ 11.96 hrs, Volume= 0.052 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Are	a (ac)	CN	Desc	cription					
	0.943	39	>75%	6 Grass co	over, Good,	HSG A			
	0.943		100.	00% Pervi	ous Area				
To (min	c Leng) (fee	th et)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
2.′	6	60 (0.3333	0.49		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-K10: SC-K10


Summary for Subcatchment SC-K11: SC-K11

Runoff = 0.26 cfs @ 11.95 hrs, Volume= 0.016 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area (a	c) C	N Dese	cription					
0.28	35 3	9 > 759	% Grass co	over, Good,	HSG A			
0.28	35	100.	00% Pervi	ous Area				
Tc L	ength	Slope	Velocity	Capacity	Description			
 1.8	<u>(1881)</u> 51	0.3333	0.47	(013)	Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-K11: SC-K11



Summary for Subcatchment SC-K12: SC-K12

Runoff = 0.85 cfs @ 11.90 hrs, Volume= 0.034 af, Depth= 2.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

A	rea (sf)	CN	Description						
	2,632	39	>75% Gras	75% Grass cover, Good, HSG A					
	1,545	76	Gravel road	ravel roads, HSG A					
	2,330	83	Paved road	ved roads w/open ditches, 50% imp, HSG A					
	6,507	64	Weighted A	verage					
	5,342		82.10% Pei	rvious Area					
	1,165		17.90% Imp	pervious Ar	ea				
Tc (min)	Length (feet)	Slope (ft/ft	e Velocity) (ft/sec)	Capacity (cfs)	Description				
0.3	41	0.146	3 2.63		Sheet Flow, Smooth surfaces n= 0.011 P2= 3.40"				

Subcatchment SC-K12: SC-K12



Summary for Subcatchment SC-K13: SC-K13

Runoff = 0.37 cfs @ 11.95 hrs, Volume= 0.023 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area	(ac) C	N Des	scription						
0.	411 3	39 >75	5% Grass c	over, Good,	, HSG A				
0.	0.411 100.00% Pervious Area								
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
1.7	46	0.3333	0.46		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"		

Subcatchment SC-K13: SC-K13



Summary for Subcatchment SC-K14: SC-K14

Runoff = 1.66 cfs @ 11.90 hrs, Volume= 0.068 af, Depth= 3.25"



Summary for Subcatchment SC-K2: SC-K2

Runoff = 0.80 cfs @ 12.06 hrs, Volume= 0.077 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area (ac) C	N Desc	cription			
1.3	397 3	39 > 759	% Grass co	over, Good,	, HSG A	
1.3	397	100.	00% Pervi	ous Area		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
7.6	100	0.0350	0.22		Sheet Flow,	
2.0	156	0.0350	1.31		Grass: Short n= 0.150 P2= 3.40" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps	
9.6	256	Total				

Subcatchment SC-K2: SC-K2



Summary for Subcatchment SC-K3: SC-K3

Runoff = 0.33 cfs @ 11.95 hrs, Volume= 0.020 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area (ac)	CN	Desc	ription					
0.3	370	39	>75%	6 Grass co	over, Good,	HSG A			
0.3	370		100.0	00% Pervi	ous Area				
Tc (min)	Length (feet)		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
1.4	38	6 0.	3333	0.45		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-K3: SC-K3



Summary for Subcatchment SC-K4: SC-K4

Runoff = 0.44 cfs @ 12.02 hrs, Volume= 0.036 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area	(ac) C	N Des	cription					
0.	661 🕄	39 >75	% Grass c	over, Good,	, HSG A			
0.	661	100	.00% Pervi	ous Area				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
6.9	89	0.0350	0.21		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-K4: SC-K4



. ..

Summary for Subcatchment SC-K5: SC-K5

Runoff = 0.61 cfs @ 11.96 hrs, Volume= 0.038 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

	AI	rea (st)	CN	Description					
		29,913	39	>75% Gras	s cover, Go	od, HSG A			
		29,913		100.00% Pe	ervious Are	а			
(r	Tc nin)	Length (feet)	Slope (ft/ft	e Velocity) (ft/sec)	Capacity (cfs)	Description			
	2.1	61	0.3333	3 0.49		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-K5: SC-K5



Summary for Subcatchment SC-K6: SC-K6

Runoff = 0.84 cfs @ 12.00 hrs, Volume= 0.061 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area	(ac) C	N Des	cription			
1.	108 3	39 >759	% Grass co	over, Good,	, HSG A	
1.	108	100.	00% Pervi	ous Area		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
4.6	100	0.1250	0.36		Sheet Flow,	
0.1	25	0.3333	4.04		Grass: Short n= 0.150 P2= 3.40" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps	
47	125	Total				

Subcatchment SC-K6: SC-K6



Summary for Subcatchment SC-K7: SC-K7

Runoff = 0.56 cfs @ 11.96 hrs, Volume= 0.035 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Are	ea (ac) C	N Des	cription					
	0.63	13	9 >75°	% Grass co	over, Good,	, HSG A			
	0.63	1	100.	00% Pervi	ous Area				
T (mir	່cLe າ) (ength (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
2.	1	61	0.3333	0.49		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-K7: SC-K7



Summary for Subcatchment SC-K8: SC-K8

Runoff = 0.97 cfs @ 11.96 hrs, Volume= 0.060 af, Depth= 0.66"



Summary for Subcatchment SC-K9: SC-K9

Runoff = 0.51 cfs @ 11.96 hrs, Volume= 0.031 af, Depth= 0.66"

Are	a (ac)	CN	Desc	cription						
0.571 39 >75% Grass cover, Good, HSG A										
0.571 100.00% Pervious Area										
T (min	c Leng) (fe	jth et)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
2.	1	61	0.3333	0.49		Sheet Flow, Grass: Short n= 0	.150	P2= 3.40"		
Subcatchment SC-K9: SC-K9										



Summary for Subcatchment SC-NE1: SC-NE1

Runoff = 0.45 cfs @ 11.94 hrs, Volume= 0.023 af, Depth= 0.87"



Summary for Subcatchment SC-NE2: SC-NE2

5.35 cfs @ 11.89 hrs, Volume= 0.268 af, Depth= 6.44" Runoff =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area (ac)	CN	Description
0.500	98	Water Surface, 0% imp, HSG A
0.500		100.00% Pervious Area

Subcatchment SC-NE2: SC-NE2



Summary for Subcatchment SC-NE3: SC-NE3

Runoff = 1.19 cfs @ 11.95 hrs, Volume= 0.055 af, Depth= 1.41"

 Area (ac)	CN	Desc	ription						
0.3	358	39	>75%	5% Grass cover, Good, HSG A						
0.0)44	76	Grav	avel roads, HSG A						
 0.0	070	83	Pave	d roads w	open ditch/	nes, 50% imp, HSG A				
 0.4	172	49	Weig	hted Aver	age					
0.4	137		92.58	3% Pervio	us Area					
0.0	035		7.429	% Impervio	ous Area					
Тс	Lengt	า เร	Slope	Velocity	Capacity	Description				
 (min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
3.6	78	B 0.	1410	0.36		Sheet Flow,				
						Grass: Short n= 0.150 P2= 3.40"				





Summary for Subcatchment SC-NE4: SC-NE4

4.95 cfs @ 11.89 hrs, Volume= 0.248 af, Depth= 6.44" Runoff =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area (ac)	CN	Description
0.462	98	Water Surface, 0% imp, HSG A
0.462		100.00% Pervious Area

Subcatchment SC-NE4: SC-NE4



Summary for Subcatchment SC-NE5: SC-NE5

Runoff = 7.68 cfs @ 11.97 hrs, Volume= 0.486 af, Depth= 0.72"



Summary for Subcatchment SC-SW1: SC-SW1

Runoff = 0.15 cfs @ 11.95 hrs, Volume= 0.009 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

A	rea (sf)	CN D	Description					
	7,247	39 >	75% Gras	s cover, Go	od, HSG A			
	7,247	1	00.00% Pe	ervious Are	а			
Tc (min)	Length	Slope	Velocity	Capacity	Description			
1.7	<u>(1881)</u> 41	0.2439	0.40	(013)	Sheet Flow.			
		0	•••••		Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-SW1: SC-SW1



Summary for Subcatchment SC-SW2: SC-SW2

Runoff = 4.26 cfs @ 11.89 hrs, Volume= 0.213 af, Depth= 6.44"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

 Area (sf)	CN	Description
17,319	98	Water Surface, 0% imp, HSG A
17,319		100.00% Pervious Area

Subcatchment SC-SW2: SC-SW2



Summary for Subcatchment SC-SW3: SC-SW3

Runoff = 0.40 cfs @ 11.94 hrs, Volume= 0.024 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area ((ac) (CN	Desc	ription					
0.4	447	39	>75%	6 Grass co	over, Good,	HSG A			
0.4	447		100.0	00% Pervi	ous Area				
Tc (min)	Length (feet)	S	lope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
0.8	15	0.2	2000	0.30		Sheet Flow, Grass: Short	n= 0.150	P2= 3.40"	

Subcatchment SC-SW3: SC-SW3



Summary for Subcatchment SC-SW4: SC-SW4

Runoff = 2.87 cfs @ 11.92 hrs, Volume= 0.175 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr,24-hr Rainfall=6.68"

Area (ac)	CN	Description
3.197	39	>75% Grass cover, Good, HSG A
3.197		100.00% Pervious Area

Subcatchment SC-SW4: SC-SW4



Summary for Reach C-1: C-1

 Inflow Area =
 30.159 ac, 1.01% Impervious, Inflow Depth = 0.79" for 25-yr,24-hr event

 Inflow =
 12.08 cfs @
 12.17 hrs, Volume=
 1.988 af

 Outflow =
 12.03 cfs @
 12.17 hrs, Volume=
 1.988 af, Atten= 0%, Lag= 0.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 6.75 fps, Min. Travel Time= 0.2 min Avg. Velocity = 2.79 fps, Avg. Travel Time= 0.6 min

Peak Storage= 179 cf @ 12.17 hrs Average Depth at Peak Storage= 1.11' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 20.38 cfs

24.0" Round Pipe n= 0.025 Corrugated metal Length= 100.0' Slope= 0.0300 '/' Inlet Invert= 11.80', Outlet Invert= 8.80'





Reach C-1: C-1

Summary for Reach C-2: C-2

 Inflow Area =
 12.471 ac, 0.00% Impervious, Inflow Depth = 0.78" for 25-yr,24-hr event

 Inflow =
 4.12 cfs @
 12.01 hrs, Volume=
 0.815 af

 Outflow =
 4.10 cfs @
 12.01 hrs, Volume=
 0.815 af, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 12.65 fps, Min. Travel Time= 0.1 min Avg. Velocity = 5.88 fps, Avg. Travel Time= 0.2 min

Peak Storage= 24 cf @ 12.01 hrs Average Depth at Peak Storage= 0.29' Bank-Full Depth= 2.50' Flow Area= 4.9 sf, Capacity= 139.45 cfs

30.0" Round Pipe n= 0.012 Concrete pipe, finished Length= 73.0' Slope= 0.0985 '/' Inlet Invert= 23.24', Outlet Invert= 16.05'





Reach C-2: C-2

Summary for Reach LP-10A: LP-10A

 Inflow Area =
 2.809 ac, 0.00% Impervious, Inflow Depth = 0.66" for 25-yr,24-hr event

 Inflow =
 1.09 cfs @
 12.17 hrs, Volume=
 0.154 af

 Outflow =
 1.09 cfs @
 12.17 hrs, Volume=
 0.154 af, Atten= 0%, Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 11.74 fps, Min. Travel Time= 0.1 min Avg. Velocity = 6.05 fps, Avg. Travel Time= 0.2 min

Peak Storage= 6 cf @ 12.17 hrs Average Depth at Peak Storage= 0.14' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 118.18 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 68.7' Slope= 0.2729 '/' Inlet Invert= 89.61', Outlet Invert= 70.86'



Hydrograph Inflow Outflow 1 09 cfs 1.09 cfs Inflow Area=2.809 ac Avg. Flow Depth=0.14' 1 Max Vel=11.74 fps 24.0" **Round Pipe** Flow (cfs) n=0.013 L=68.7' S=0.2729 '/' Capacity=118.18 cfs 8 10 12 14 16 18 Ò Ż 4 6 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Time (hours)

Reach LP-10A: LP-10A

Summary for Reach LP-10B: LP-10B

 Inflow Area =
 2.953 ac, 0.00% Impervious, Inflow Depth = 0.66" for 25-yr,24-hr event

 Inflow =
 1.11 cfs @
 12.17 hrs, Volume=
 0.162 af

 Outflow =
 1.10 cfs @
 12.18 hrs, Volume=
 0.162 af, Atten= 1%, Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 11.76 fps, Min. Travel Time= 0.1 min Avg. Velocity = 6.09 fps, Avg. Travel Time= 0.2 min

Peak Storage= 7 cf @ 12.17 hrs Average Depth at Peak Storage= 0.14' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 117.34 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 70.1' Slope= 0.2690 '/' Inlet Invert= 70.86', Outlet Invert= 52.00'



Hydrograph Inflow Outflow 1.11 cfs 1.10 cfs Inflow Area=2.953 ac Avg. Flow Depth=0.14' 1 Max Vel=11.76 fps 24.0" **Round Pipe** Flow (cfs) n=0.013 L=70.1' S=0.2690 '/' Capacity=117.34 cfs Ò Ż 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Time (hours)

Reach LP-10B: LP-10B

Summary for Reach LP-10C: LP-10C

 Inflow Area =
 3.244 ac,
 0.00% Impervious, Inflow Depth =
 0.66" for 25-yr,24-hr event

 Inflow =
 1.20 cfs @
 12.17 hrs, Volume=
 0.178 af

 Outflow =
 1.20 cfs @
 12.17 hrs, Volume=
 0.178 af, Atten= 0%, Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 12.14 fps, Min. Travel Time= 0.1 min Avg. Velocity = 6.30 fps, Avg. Travel Time= 0.2 min

Peak Storage= 7 cf @ 12.17 hrs Average Depth at Peak Storage= 0.14' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 118.46 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 70.1' Slope= 0.2742 '/' Inlet Invert= 52.00', Outlet Invert= 32.78'



Hydrograph Inflow Outflow 1 20 cfs 1.20 cfs Inflow Area=3.244 ac Avg. Flow Depth=0.14' Max Vel=12.14 fps 1 24.0" **Round Pipe** Flow (cfs) n=0.013 L=70.1' S=0.2742 '/' Capacity=118.46 cfs Ò Ż 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Time (hours)

Reach LP-10C: LP-10C

Summary for Reach LP-10D: LP-10D

 Inflow Area =
 3.643 ac,
 0.00% Impervious, Inflow Depth =
 0.66" for 25-yr,24-hr event

 Inflow =
 1.35 cfs @
 12.15 hrs, Volume=
 0.200 af

 Outflow =
 1.35 cfs @
 12.15 hrs, Volume=
 0.200 af, Atten= 0%, Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 13.09 fps, Min. Travel Time= 0.1 min Avg. Velocity = 6.76 fps, Avg. Travel Time= 0.2 min

Peak Storage= 6 cf @ 12.15 hrs Average Depth at Peak Storage= 0.15' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 124.87 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 61.7' Slope= 0.3047 '/' Inlet Invert= 32.78', Outlet Invert= 13.98'



Hydrograph Inflow Outflow 1 35 cfs 1.35 cfs Inflow Area=3.643 ac Avg. Flow Depth=0.15' Max Vel=13.09 fps 24.0" **Round Pipe** Flow (cfs) n=0.013 L=61.7' S=0.3047 '/' Capacity=124.87 cfs Ò Ż 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Time (hours)

Reach LP-10D: LP-10D

Summary for Reach LP-11A: LP-11A

 Inflow Area =
 4.198 ac, 0.00% Impervious, Inflow Depth = 0.66" for 25-yr,24-hr event

 Inflow =
 1.59 cfs @
 12.21 hrs, Volume=
 0.230 af

 Outflow =
 1.59 cfs @
 12.21 hrs, Volume=
 0.230 af, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 11.75 fps, Min. Travel Time= 0.0 min Avg. Velocity = 5.92 fps, Avg. Travel Time= 0.1 min

Peak Storage= 4 cf @ 12.21 hrs Average Depth at Peak Storage= 0.18' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 99.94 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 30.9' Slope= 0.1951 '/' Inlet Invert= 90.05', Outlet Invert= 84.02'



Hydrograph Inflow Outflow 1 59 cfs 1.59 cfs Inflow Area=4.198 ac Avg. Flow Depth=0.18' Max Vel=11.75 fps 24.0" **Round Pipe** Flow (cfs) n=0.013 L=30.9' S=0.1951 '/' Capacity=99.94 cfs 10 12 14 16 18 Ò Ż 4 6 8 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Time (hours)

Reach LP-11A: LP-11A

Summary for Reach LP-11B: LP-11B

 Inflow Area =
 5.229 ac,
 0.00% Impervious,
 Inflow Depth =
 0.66"
 for 25-yr,24-hr event

 Inflow =
 2.00 cfs @
 12.20 hrs,
 Volume=
 0.286 af

 Outflow =
 2.00 cfs @
 12.20 hrs,
 Volume=
 0.286 af,

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 14.15 fps, Min. Travel Time= 0.1 min Avg. Velocity = 7.02 fps, Avg. Travel Time= 0.2 min

Peak Storage= 10 cf @ 12.20 hrs Average Depth at Peak Storage= 0.18' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 117.86 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 70.0' Slope= 0.2714 '/' Inlet Invert= 84.00', Outlet Invert= 65.00'



Hydrograph Inflow Outflow 2 00 cfs 2.00 cfs Inflow Area=5.229 ac 2 Avg. Flow Depth=0.18' Max Vel=14.15 fps 24.0" **Round Pipe** Flow (cfs) n=0.013 L=70.0' S=0.2714 '/' Capacity=117.86 cfs 10 12 14 16 18 Ò Ż 4 6 8 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Time (hours)

Reach LP-11B: LP-11B

Summary for Reach LP-11C: LP-11C

 Inflow Area =
 7.024 ac, 0.00% Impervious, Inflow Depth = 0.66" for 25-yr,24-hr event

 Inflow =
 2.71 cfs @
 12.19 hrs, Volume=
 0.385 af

 Outflow =
 2.70 cfs @
 12.19 hrs, Volume=
 0.385 af, Atten= 0%, Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 15.39 fps, Min. Travel Time= 0.1 min Avg. Velocity = 7.45 fps, Avg. Travel Time= 0.2 min

Peak Storage= 13 cf @ 12.19 hrs Average Depth at Peak Storage= 0.21' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 116.74 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 71.2' Slope= 0.2663 '/' Inlet Invert= 65.00', Outlet Invert= 46.04'



Hydrograph Inflow Outflow 2 71 cfs 3 2.70 cfs Inflow Area=7.024 ac Avg. Flow Depth=0.21' Max Vel=15.39 fps 24.0" 2 **Round Pipe** Flow (cfs) n=0.013 L=71.2' S=0.2663 '/' Capacity=116.74 cfs Ò Ż 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Time (hours)

Reach LP-11C: LP-11C

Summary for Reach LP-11D: LP-11D

 Inflow Area =
 8.744 ac, 0.00% Impervious, Inflow Depth = 0.66" for 25-yr,24-hr event

 Inflow =
 3.36 cfs @
 12.18 hrs, Volume=
 0.479 af

 Outflow =
 3.35 cfs @
 12.18 hrs, Volume=
 0.479 af, Atten= 0%, Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 16.08 fps, Min. Travel Time= 0.1 min Avg. Velocity = 7.67 fps, Avg. Travel Time= 0.2 min

Peak Storage= 15 cf @ 12.18 hrs Average Depth at Peak Storage= 0.24' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 113.63 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 71.7' Slope= 0.2523 '/' Inlet Invert= 46.00', Outlet Invert= 27.91'



Hydrograph Inflow Outflow <u>3 36 cfs</u> 3.35 cfs Inflow Area=8.744 ac Avg. Flow Depth=0.24' 3 Max Vel=16.08 fps 24.0" **Round Pipe** Flow (cfs) 2 n=0.013 L=71.7' S=0.2523 '/' 1 Capacity=113.63 cfs 10 12 14 16 18 Ò Ż 4 6 8 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Time (hours)

Reach LP-11D: LP-11D

Summary for Reach LP-11E: LP-11E

 Inflow Area =
 10.258 ac, 0.00% Impervious, Inflow Depth = 0.66" for 25-yr,24-hr event

 Inflow =
 4.01 cfs @
 12.17 hrs, Volume=
 0.562 af

 Outflow =
 4.00 cfs @
 12.17 hrs, Volume=
 0.562 af, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 18.42 fps, Min. Travel Time= 0.0 min Avg. Velocity = 8.73 fps, Avg. Travel Time= 0.1 min

Peak Storage= 11 cf @ 12.17 hrs Average Depth at Peak Storage= 0.24' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 128.78 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 49.1' Slope= 0.3240 '/' Inlet Invert= 27.91', Outlet Invert= 12.00'



Hydrograph Inflow Outflow 4 01 cfs 4.00 cfs Inflow Area=10.258 ac 4 Avg. Flow Depth=0.24' Max Vel=18.42 fps 24.0" 3 **Round Pipe** Flow (cfs) n=0.013 2 L=49.1' S=0.3240 '/' Capacity=128.78 cfs Ò Ż 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Time (hours)

Reach LP-11E: LP-11E

Summary for Reach LP-5A: LP-5A

 Inflow Area =
 5.738 ac, 0.00% Impervious, Inflow Depth = 0.66" for 25-yr,24-hr event

 Inflow =
 1.80 cfs @
 12.27 hrs, Volume=
 0.314 af

 Outflow =
 1.80 cfs @
 12.27 hrs, Volume=
 0.314 af, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 14.70 fps, Min. Travel Time= 0.0 min Avg. Velocity = 7.93 fps, Avg. Travel Time= 0.1 min

Peak Storage= 4 cf @ 12.27 hrs Average Depth at Peak Storage= 0.16' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 130.57 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 34.7' Slope= 0.3331 '/' Inlet Invert= 90.00', Outlet Invert= 78.44'



Hydrograph Inflow Outflow 1 80 cfs 2 1.80 cfs Inflow Area=5.738 ac Avg. Flow Depth=0.16' Max Vel=14.70 fps 24.0" **Round Pipe** Flow (cfs) n=0.013 L=34.7' S=0.3331 '/' Capacity=130.57 cfs 8 10 12 14 16 18 Ò Ż 4 6 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Time (hours)

Reach LP-5A: LP-5A

Summary for Reach LP-5B: LP-5B

 Inflow Area =
 5.888 ac, 0.00% Impervious, Inflow Depth = 0.66" for 25-yr,24-hr event

 Inflow =
 1.83 cfs @
 12.27 hrs, Volume=
 0.323 af

 Outflow =
 1.83 cfs @
 12.27 hrs, Volume=
 0.323 af, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 13.77 fps, Min. Travel Time= 0.1 min Avg. Velocity = 7.44 fps, Avg. Travel Time= 0.2 min

Peak Storage= 9 cf @ 12.27 hrs Average Depth at Peak Storage= 0.17' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 118.05 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 70.4' Slope= 0.2723 '/' Inlet Invert= 78.44', Outlet Invert= 59.27'



Hydrograph Inflow Outflow 1 83 cfs 2 1.83 cfs Inflow Area=5.888 ac Avg. Flow Depth=0.17' Max Vel=13.77 fps 24.0" **Round Pipe** Flow (cfs) n=0.013 L=70.4' S=0.2723 '/' Capacity=118.05 cfs Ò Ż 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Time (hours)

Reach LP-5B: LP-5B

Summary for Reach LP-5C: LP-5C

 Inflow Area =
 6.201 ac,
 0.00% Impervious,
 Inflow Depth =
 0.66"
 for
 25-yr,24-hr event

 Inflow =
 1.90 cfs @
 12.27 hrs,
 Volume=
 0.340 af

 Outflow =
 1.90 cfs @
 12.27 hrs,
 Volume=
 0.340 af,

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 13.78 fps, Min. Travel Time= 0.1 min Avg. Velocity = 7.48 fps, Avg. Travel Time= 0.1 min

Peak Storage= 9 cf @ 12.27 hrs Average Depth at Peak Storage= 0.18' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 116.53 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 63.5' Slope= 0.2654 '/' Inlet Invert= 59.27', Outlet Invert= 42.42'



Hydrograph Inflow Outflow 1 90 cfs 1.90 cfs 2 Inflow Area=6.201 ac Avg. Flow Depth=0.18' Max Vel=13.78 fps 24.0" **Round Pipe** Flow (cfs) n=0.013 L=63.5' S=0.2654 '/' Capacity=116.53 cfs 10 12 14 16 18 Ò Ż 4 6 8 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Time (hours)

Reach LP-5C: LP-5C

Summary for Reach LP-5D: LP-5D

 Inflow Area =
 6.586 ac,
 0.00% Impervious,
 Inflow Depth =
 0.66"
 for 25-yr,24-hr event

 Inflow =
 1.99 cfs @
 12.27 hrs,
 Volume=
 0.361 af

 Outflow =
 1.99 cfs @
 12.27 hrs,
 Volume=
 0.361 af,

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 13.40 fps, Min. Travel Time= 0.1 min Avg. Velocity = 7.26 fps, Avg. Travel Time= 0.1 min

Peak Storage= 9 cf @ 12.27 hrs Average Depth at Peak Storage= 0.19' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 109.43 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 57.4' Slope= 0.2340 '/' Inlet Invert= 42.42', Outlet Invert= 28.99'



Hydrograph Inflow Outflow 1 99 cfs 1.99 cfs Inflow Area=6.586 ac 2 Avg. Flow Depth=0.19' Max Vel=13.40 fps 24.0" **Round Pipe** Flow (cfs) n=0.013 L=57.4' S=0.2340 '/' Capacity=109.43 cfs 10 12 14 16 18 Ò Ż 4 6 8 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Time (hours)

Reach LP-5D: LP-5D
Summary for Reach LP-6A: LP-6A

 Inflow Area =
 0.967 ac,
 0.00% Impervious,
 Inflow Depth =
 0.66"
 for 25-yr,24-hr event

 Inflow =
 0.34 cfs @
 12.25 hrs,
 Volume=
 0.053 af

 Outflow =
 0.34 cfs @
 12.26 hrs,
 Volume=
 0.053 af,

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 7.34 fps, Min. Travel Time= 0.2 min Avg. Velocity = 3.86 fps, Avg. Travel Time= 0.5 min

Peak Storage= 5 cf @ 12.25 hrs Average Depth at Peak Storage= 0.09' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 98.84 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 110.0' Slope= 0.1909 '/' Inlet Invert= 73.00', Outlet Invert= 52.00'



Hydrograph Inflow Outflow 0.34 cfs 0.36 0.34 cfs Inflow Area=0.967 ac 0.34 0.32 Avg. Flow Depth=0.09' 0.3 Max Vel=7.34 fps 0.28 0.26 24.0" 0.24 **Round Pipe** 0.22 (cfs) 0.2 n=0.013 Flow 0.18 0.16 L=110.0' 0.14 S=0.1909 '/' 0.12 0.1 Capacity=98.84 cfs 0.08 0.06 0.04 0.02 0 Ż 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 0 Time (hours)

Reach LP-6A: LP-6A

Summary for Reach LP-6B: LP-6B

 Inflow Area =
 2.183 ac, 0.00% Impervious, Inflow Depth = 0.66" for 25-yr,24-hr event

 Inflow =
 0.76 cfs @
 12.25 hrs, Volume=
 0.120 af

 Outflow =
 0.76 cfs @
 12.25 hrs, Volume=
 0.120 af, Atten= 0%, Lag= 0.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 9.27 fps, Min. Travel Time= 0.2 min Avg. Velocity = 4.60 fps, Avg. Travel Time= 0.4 min

Peak Storage= 9 cf @ 12.25 hrs Average Depth at Peak Storage= 0.13' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 97.44 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 113.2' Slope= 0.1855 '/' Inlet Invert= 52.00', Outlet Invert= 31.00'



Hydrograph Inflow Outflow 0.85 0 76 cfs 0.76 cfs 0.8 Inflow Area=2.183 ac 0.75 Avg. Flow Depth=0.13' 0.7 Max Vel=9.27 fps 0.65 0.6 24.0" 0.55 **Round Pipe** 0.5 (sj) 0.45 n=0.013 Flow 0.4 L=113.2' 0.35 03 S=0.1855 '/' 0.25 Capacity=97.44 cfs 0.2 0.15 0.1 0.05 0 Ò Ż 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Time (hours)

Reach LP-6B: LP-6B

Summary for Reach LP-6C: LP-6C

 Inflow Area =
 3.540 ac,
 0.00% Impervious, Inflow Depth =
 0.66" for 25-yr,24-hr event

 Inflow =
 1.21 cfs @
 12.26 hrs, Volume=
 0.194 af

 Outflow =
 1.21 cfs @
 12.26 hrs, Volume=
 0.194 af,

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 10.30 fps, Min. Travel Time= 0.1 min Avg. Velocity = 5.01 fps, Avg. Travel Time= 0.2 min

Peak Storage= 6 cf @ 12.26 hrs Average Depth at Peak Storage= 0.16' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 92.94 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 47.4' Slope= 0.1688 '/' Inlet Invert= 31.00', Outlet Invert= 23.00'



Hydrograph Inflow Outflow 1 21 cfs 1.21 cfs Inflow Area=3.540 ac Avg. Flow Depth=0.16' Max Vel=10.30 fps 1 24.0" **Round Pipe** Flow (cfs) n=0.013 L=47.4' S=0.1688 '/' Capacity=92.94 cfs Ò Ż 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Time (hours)

Reach LP-6C: LP-6C

Summary for Reach LP-7A: LP-7A

 Inflow Area =
 1.692 ac, 0.00% Impervious, Inflow Depth = 0.66" for 25-yr,24-hr event

 Inflow =
 0.77 cfs @
 12.18 hrs, Volume=
 0.093 af

 Outflow =
 0.77 cfs @
 12.18 hrs, Volume=
 0.093 af, Atten= 0%, Lag= 0.3 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 10.64 fps, Min. Travel Time= 0.1 min Avg. Velocity = 5.26 fps, Avg. Travel Time= 0.2 min

Peak Storage= 5 cf @ 12.18 hrs Average Depth at Peak Storage= 0.11' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 119.14 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 68.5' Slope= 0.2774 '/' Inlet Invert= 79.00', Outlet Invert= 60.00'



Reach LP-7A: LP-7A



Summary for Reach LP-7B: LP-7B

 Inflow Area =
 2.528 ac, 0.00% Impervious, Inflow Depth = 0.66" for 25-yr,24-hr event

 Inflow =
 1.08 cfs @
 12.16 hrs, Volume=
 0.138 af

 Outflow =
 1.08 cfs @
 12.16 hrs, Volume=
 0.138 af, Atten= 0%, Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 11.75 fps, Min. Travel Time= 0.1 min Avg. Velocity = 5.77 fps, Avg. Travel Time= 0.2 min

Peak Storage= 6 cf @ 12.16 hrs Average Depth at Peak Storage= 0.14' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 117.78 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 70.1' Slope= 0.2710 '/' Inlet Invert= 60.00', Outlet Invert= 41.00'



Hydrograph Inflow Outflow 1 08 cfs 1.08 cfs Inflow Area=2,528 ac Avg. Flow Depth=0.14' 1 Max Vel=11.75 fps 24.0" **Round Pipe** Flow (cfs) n=0.013 L=70.1' S=0.2710 '/' Capacity=117.78 cfs 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Ò Ż 4 6 8 Time (hours)

Reach LP-7B: LP-7B

Summary for Reach LP-7C: LP-7C

 Inflow Area =
 3.376 ac,
 0.00% Impervious, Inflow Depth =
 0.66" for 25-yr,24-hr event

 Inflow =
 1.44 cfs @
 12.13 hrs, Volume=
 0.185 af

 Outflow =
 1.43 cfs @
 12.14 hrs, Volume=
 0.185 af, Atten= 0%, Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 12.25 fps, Min. Travel Time= 0.1 min Avg. Velocity = 5.98 fps, Avg. Travel Time= 0.1 min

Peak Storage= 6 cf @ 12.13 hrs Average Depth at Peak Storage= 0.16' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 110.97 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 49.5' Slope= 0.2406 '/' Inlet Invert= 41.00', Outlet Invert= 29.09'



Reach LP-7C: LP-7C



Summary for Reach LP-8A: LP-8A

 Inflow Area =
 0.537 ac,
 0.00% Impervious,
 Inflow Depth =
 0.66"
 for 25-yr,24-hr event

 Inflow =
 0.24 cfs @
 12.16 hrs,
 Volume=
 0.029 af

 Outflow =
 0.23 cfs @
 12.17 hrs,
 Volume=
 0.029 af,

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 6.44 fps, Min. Travel Time= 0.2 min Avg. Velocity = 3.23 fps, Avg. Travel Time= 0.5 min

Peak Storage= 3 cf @ 12.16 hrs Average Depth at Peak Storage= 0.07' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 95.85 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 94.7' Slope= 0.1795 '/' Inlet Invert= 76.00', Outlet Invert= 59.00'



Hydrograph Inflow Outflow 0.24 cfs 0.26 0.23 cfs Inflow Area=0.537 ac 0.24 Avg. Flow Depth=0.07' 0 22 Max Vel=6.44 fps 0.2 0.18 24.0" 0.16 **Round Pipe** (**sj**) 0.14 n=0.013 _low 0.12 L=94.7' 0 1 S=0.1795 '/' 0.08 Capacity=95.85 cfs 0.06 0.04 0.02 0 10 12 14 16 18 20 Ż 4 6 8 22 24 26 28 30 32 34 36 38 40 42 44 46 48 0 Time (hours)

Reach LP-8A: LP-8A

Summary for Reach LP-8B: LP-8B

 Inflow Area =
 1.315 ac, 0.00% Impervious, Inflow Depth = 0.66" for 25-yr,24-hr event

 Inflow =
 0.61 cfs @
 12.15 hrs, Volume=
 0.072 af

 Outflow =
 0.61 cfs @
 12.15 hrs, Volume=
 0.072 af, Atten= 0%, Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 7.76 fps, Min. Travel Time= 0.1 min Avg. Velocity = 3.71 fps, Avg. Travel Time= 0.2 min

Peak Storage= 4 cf @ 12.15 hrs Average Depth at Peak Storage= 0.12' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 83.16 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 44.4' Slope= 0.1351 '/' Inlet Invert= 59.00', Outlet Invert= 53.00'



Hydrograph Inflow Outflow 0 61 cfs 0.65 0.61 cfs Inflow Area=1.315 ac 0.6 Avg. Flow Depth=0.12' 0.55 Max Vel=7.76 fps 0.5 24.0" 0.45 **Round Pipe** 0.4 Flow (cfs) 0.35 n=0.013 0.3 L=44.4' 0.25 S=0.1351 '/' 0.2 Capacity=83.16 cfs 0.15 0.1 0.05 0 10 12 14 16 18 20 Ż 4 6 8 22 24 26 28 30 32 34 36 38 40 42 44 46 48 0 Time (hours)

Reach LP-8B: LP-8B

Summary for Reach LP-8C: LP-8C

Inflow Area = 2.139 ac. 0.00% Impervious, Inflow Depth = 0.66" for 25-yr,24-hr event Inflow 0.97 cfs @ 12.16 hrs, Volume= 0.117 af = Outflow 0.97 cfs @ 12.16 hrs, Volume= = 0.117 af, Atten= 0%, Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 9.21 fps, Min. Travel Time= 0.1 min Avg. Velocity = 4.32 fps, Avg. Travel Time= 0.2 min

Peak Storage= 6 cf @ 12.16 hrs Average Depth at Peak Storage= 0.15' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 87.11 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 60.7' Šlope= 0.1483 '/' Inlet Invert= 53.00', Outlet Invert= 44.00'



Hydrograph Inflow Outflow 0 97 cfs 0.97 cfs Inflow Area=2.139 ac Avg. Flow Depth=0.15' Max Vel=9.21 fps 24.0" **Round Pipe** Flow (cfs) n=0.013 L=60.7' S=0.1483 '/' Capacity=87.11 cfs 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Ò Ż 4 6 Time (hours)

Reach LP-8C: LP-8C

Summary for Reach LP-8D: LP-8D

 Inflow Area =
 2.848 ac, 0.00% Impervious, Inflow Depth = 0.66" for 25-yr,24-hr event

 Inflow =
 1.27 cfs @
 12.16 hrs, Volume=
 0.156 af

 Outflow =
 1.27 cfs @
 12.16 hrs, Volume=
 0.156 af, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 11.61 fps, Min. Travel Time= 0.1 min Avg. Velocity = 5.46 fps, Avg. Travel Time= 0.1 min

Peak Storage= 5 cf @ 12.16 hrs Average Depth at Peak Storage= 0.15' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 108.30 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 48.0' Slope= 0.2292 '/' Inlet Invert= 44.00', Outlet Invert= 33.00'



Hydrograph Inflow Outflow 1 27 cfs 1.27 cfs Inflow Area=2.848 ac Avg. Flow Depth=0.15' Max Vel=11.61 fps 1 24.0" **Round Pipe** Flow (cfs) n=0.013 L=48.0' S=0.2292 '/' Capacity=108.30 cfs Ò Ż 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Time (hours)

Reach LP-8D: LP-8D

Summary for Reach LP-8E: LP-8E

 Inflow Area =
 3.825 ac,
 0.00% Impervious,
 Inflow Depth =
 0.66"
 for 25-yr,24-hr event

 Inflow =
 1.67 cfs @
 12.17 hrs,
 Volume=
 0.210 af

 Outflow =
 1.67 cfs @
 12.17 hrs,
 Volume=
 0.210 af,

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 12.40 fps, Min. Travel Time= 0.1 min Avg. Velocity = 5.81 fps, Avg. Travel Time= 0.1 min

Peak Storage= 6 cf @ 12.17 hrs Average Depth at Peak Storage= 0.18' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 106.17 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 45.4' Slope= 0.2203 '/' Inlet Invert= 33.00', Outlet Invert= 23.00'



Hydrograph Inflow Outflow 1 67 cfs 1.67 cfs Inflow Area=3.825 ac Avg. Flow Depth=0.18' Max Vel=12.40 fps 24.0" **Round Pipe** Flow (cfs) n=0.013 L=45.4' S=0.2203 '/' Capacity=106.17 cfs Ò Ż 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Time (hours)

Reach LP-8E: LP-8E

Summary for Reach LP-8F: LP-8F

 Inflow Area =
 4.754 ac, 0.00% Impervious, Inflow Depth = 0.66" for 25-yr,24-hr event

 Inflow =
 2.03 cfs @
 12.17 hrs, Volume=
 0.260 af

 Outflow =
 2.02 cfs @
 12.17 hrs, Volume=
 0.260 af, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 10.87 fps, Min. Travel Time= 0.0 min Avg. Velocity = 5.03 fps, Avg. Travel Time= 0.1 min

Peak Storage= 6 cf @ 12.17 hrs Average Depth at Peak Storage= 0.22' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 81.50 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 30.9' Slope= 0.1298 '/' Inlet Invert= 23.00', Outlet Invert= 18.99'





Reach LP-8F: LP-8F

Summary for Reach LP-9A: LP-9A

 Inflow Area =
 1.908 ac, 0.00% Impervious, Inflow Depth = 0.66" for 25-yr,24-hr event

 Inflow =
 0.84 cfs @
 12.12 hrs, Volume=
 0.105 af

 Outflow =
 0.83 cfs @
 12.13 hrs, Volume=
 0.105 af, Atten= 2%, Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 11.52 fps, Min. Travel Time= 0.1 min Avg. Velocity = 5.84 fps, Avg. Travel Time= 0.2 min

Peak Storage= 5 cf @ 12.12 hrs Average Depth at Peak Storage= 0.12' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 129.05 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 67.7' Slope= 0.3254 '/' Inlet Invert= 90.00', Outlet Invert= 67.97'



Hydrograph Inflow Outflow 0.84 cfs 09 0.83 cfs Inflow Area=1.908 ac 0.85 0.8 Avg. Flow Depth=0.12' 0.75 Max Vel=11.52 fps 0.7 0.65 24.0" 0.6 **Round Pipe** 0.55 (cfs) 0.5 n=0.013 Flow 0.45 0.4 L=67.7' 0.35 S=0.3254 '/' 0.3 0.25 Capacity=129.05 cfs 0.2 0.15 0.1 0.05 0 Ż 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 0 Time (hours)

Reach LP-9A: LP-9A

Summary for Reach LP-9B: LP-9B

 Inflow Area =
 2.451 ac, 0.00% Impervious, Inflow Depth = 0.66" for 25-yr,24-hr event

 Inflow =
 1.12 cfs @
 12.11 hrs, Volume=
 0.134 af

 Outflow =
 1.12 cfs @
 12.12 hrs, Volume=
 0.134 af, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 11.33 fps, Min. Travel Time= 0.1 min Avg. Velocity = 5.54 fps, Avg. Travel Time= 0.2 min

Peak Storage= 6 cf @ 12.12 hrs Average Depth at Peak Storage= 0.14' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 110.69 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 59.9' Slope= 0.2394 '/' Inlet Invert= 67.97', Outlet Invert= 53.63'



Hydrograph Inflow Outflow 1 12 cfs 1.12 cfs Inflow Area=2.451 ac Avg. Flow Depth=0.14' 1 Max Vel=11.33 fps 24.0" **Round Pipe** Flow (cfs) n=0.013 L=59.9' S=0.2394 '/' Capacity=110.69 cfs 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Ò Ż 4 6 Time (hours)

Reach LP-9B: LP-9B

Summary for Reach LP-9C: LP-9C

 Inflow Area =
 2.629 ac, 0.00% Impervious, Inflow Depth = 0.66" for 25-yr,24-hr event

 Inflow =
 1.21 cfs @
 12.11 hrs, Volume=
 0.144 af

 Outflow =
 1.21 cfs @
 12.12 hrs, Volume=
 0.144 af, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 12.44 fps, Min. Travel Time= 0.1 min Avg. Velocity = 6.06 fps, Avg. Travel Time= 0.2 min

Peak Storage= 6 cf @ 12.12 hrs Average Depth at Peak Storage= 0.14' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 122.60 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 63.4' Slope= 0.2937 '/' Inlet Invert= 53.63', Outlet Invert= 35.01'



Hydrograph Inflow Outflow 1 21 cfs 1.21 cfs Inflow Area=2.629 ac Avg. Flow Depth=0.14' Max Vel=12.44 fps 1 24.0" **Round Pipe** Flow (cfs) n=0.013 L=63.4' S=0.2937 '/' Capacity=122.60 cfs 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Ò Ż 4 6 Time (hours)

Reach LP-9C: LP-9C

Summary for Reach LP-9D: LP-9D

 Inflow Area =
 2.688 ac, 0.00% Impervious, Inflow Depth = 0.66" for 25-yr,24-hr event

 Inflow =
 1.22 cfs @
 12.12 hrs, Volume=
 0.147 af

 Outflow =
 1.21 cfs @
 12.12 hrs, Volume=
 0.147 af, Atten= 1%, Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 11.99 fps, Min. Travel Time= 0.1 min Avg. Velocity = 5.85 fps, Avg. Travel Time= 0.2 min

Peak Storage= 8 cf @ 12.12 hrs Average Depth at Peak Storage= 0.14' Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 115.93 cfs

24.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 76.2' Slope= 0.2626 '/' Inlet Invert= 35.01', Outlet Invert= 15.00'



Hydrograph Inflow Outflow 1.22 cfs 1.21 cfs Inflow Area=2.688 ac Avg. Flow Depth=0.14' Max Vel=11.99 fps 1 24.0" **Round Pipe** Flow (cfs) n=0.013 L=76.2' S=0.2626 '/' Capacity=115.93 cfs Ò Ż 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Time (hours)

Reach LP-9D: LP-9D

Summary for Reach PC-10: PC-10



Summary for Reach PC-11: PC-11



Summary for Reach PC-12: PC-12



Summary for Reach PC-13: PC-13



Summary for Reach PC-14: PC-14



Summary for Reach PC-5: PC-5



Summary for Reach PC-6: PC-6



Summary for Reach PC-7: PC-7



Summary for Reach PC-8: PC-8



Summary for Reach PC-9: PC-9



Summary for Reach TB-E1: TB-E1



Summary for Reach TB-E2: TB-E2



Summary for Reach TB-E3: TB-E3



Summary for Reach TB-F1: TB-F1



Summary for Reach TB-F2: TB-F2



Summary for Reach TB-F3: TB-F3



Summary for Reach TB-G1: TB-G1



Summary for Reach TB-G2: TB-G2



Summary for Reach TB-G3: TB-G3



Summary for Reach TB-G4: TB-G4


Summary for Reach TB-G5: TB-G5



Summary for Reach TB-G6: TB-G6



Summary for Reach TB-H1: TB-H1



Summary for Reach TB-H2: TB-H2



Summary for Reach TB-H3: TB-H3



Summary for Reach TB-H4: TB-H4



Summary for Reach TB-H5: TB-H5



Summary for Reach TB-H6: TB-H6



Summary for Reach TB-I1: TB-I1



Summary for Reach TB-I2: TB-I2



Summary for Reach TB-I3: TB-I3



Summary for Reach TB-J1: TB-J1



Summary for Reach TB-J2: TB-J2



Summary for Reach TB-J3: TB-J3



Summary for Reach TB-K1: TB-K1



Summary for Reach TB-K10: TB-K10



Summary for Reach TB-K2: TB-K2



Summary for Reach TB-K3: TB-K3



Summary for Reach TB-K4: TB-K4



Summary for Reach TB-K5: TB-K5



Summary for Reach TB-K6: TB-K6



Summary for Reach TB-K7: TB-K7



Summary for Reach TB-K8: TB-K8



Summary for Reach TB-K9: TB-K9



Summary for Pond EF: East Forebay

Inflow Area =	27.531 ac,	0.00% Impervious,	Inflow Depth = 0.8	88" for 25-yr,24-hr event
Inflow =	7.65 cfs @	12.36 hrs, Volume	= 2.030 af	
Outflow =	7.17 cfs @	12.45 hrs, Volume	= 2.030 af,	Atten= 6%, Lag= 5.4 min
Primary =	7.17 cfs @	12.45 hrs, Volume	= 2.030 af	-

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Starting Elev= 7.00' Surf.Area= 21,422 sf Storage= 38,776 cf Peak Elev= 7.22' @ 12.45 hrs Surf.Area= 21,801 sf Storage= 43,625 cf (4,849 cf above start)

Plug-Flow detention time= 340.7 min calculated for 1.139 af (56% of inflow) Center-of-Mass det. time= 17.2 min (941.6 - 924.4)

Volume	In	vert Av	ail.Storage	Storage	e Description	
#1	5	.00'	111,542 cf	Custon	n Stage Data (Pi	rismatic)Listed below (Recalc)
Elevatio	on	Surf.Area	ln In	c.Store	Cum.Store	
(166	et)	(sq-π)	auo) (cub	ic-teet)	(CUDIC-TEET)	
5.0	00	16,828	1	0	0	
6.0	00	19,651		18,240	18,240	
7.0	00	21,422		20,537	38,776	
8.0	00	23,109)	22,266	61,042	
9.0	00	24,775	i	23,942	84,984	
10.0	00	28,341		26,558	111,542	
Device	Routing	g l	nvert Out	let Device	es	
#1	Primary	/	7.00' Asy Offe Hei	/mmetric set (feet) ght (feet)	al Weir, C= 3.27 0.00 9.00 29.00 3.00 0.00 0.00) 38.00 3.00
Drimony		$M_{OV} = 7.1$	2 of c @ 12	45 bro ∐	W-7 22' (Eroo I	

Primary OutFlow Max=7.13 cfs @ 12.45 hrs HW=7.22' (Free Discharge) 1=Asymmetrical Weir (Weir Controls 7.13 cfs @ 1.49 fps) Pond EF: East Forebay



Summary for Pond NE Basin: NE Basin

Inflow Area	=	66.671 ac,	0.51% Impervious	, Inflow Depth =	0.87" for	25-yr,24-hr event
Inflow	=	20.52 cfs @	11.99 hrs, Volum	e= 4.808	af	
Outflow	=	17.48 cfs @	12.48 hrs, Volum	e= 4.808	af, Atten=	15%, Lag= 29.3 min
Discarded	=	17.48 cfs @	12.48 hrs, Volum	e= 4.807	af	
Primary	=	0.00 cfs @	12.48 hrs, Volum	e= 0.000	af	

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 5.06' @ 12.48 hrs Surf.Area= 243,096 sf Storage= 14,349 cf

Plug-Flow detention time= 13.7 min calculated for 4.803 af (100% of inflow) Center-of-Mass det. time= 13.7 min (945.1 - 931.5)

Volume	١n	vert A	vail.Sto	rage	Storage	Description				
#1	5.	00'	1,399,15	54 cf	Custom	Stage Data (P	rismatic)Listed below (Recalc)			
Elevatio	on et)	Surf.Are	ea ft)	Inc (cubio	.Store c-feet)	Cum.Store (cubic-feet)				
5.0	0	241,0	17 25	25	0 1 106	254 106				
7.0	00	275,84	13	27	1,219	525,325				
8.0	00	286,09	99	28	0,971	806,296				
9.0	00	296,41	12	29	1,256	1,097,552				
10.0	00	306,79	93	30	1,603	1,399,154				
Device	Routing		Invert	Outle	et Devices	6				
#1	Primary		5.00'	0.11	3 cfs Con	stant Flow/Sk	kimmer Phase-In= 5.00'			
#2	Discard	ed	5.00'	2.97 Cond	0 in/hr Ex ductivity to	f iltration over Groundwater	^r Surface area Elevation = 4.00'			
Discard 1—2=Ex	Discarded OutFlow Max=17.70 cfs @ 12.48 hrs HW=5.06' (Free Discharge) ☐ 2=Exfiltration (Controls 17.70 cfs)									

Primary OutFlow Max=0.00 cfs @ 12.48 hrs HW=5.06' (Free Discharge) —1=Constant Flow/Skimmer (Constant Controls 0.00 cfs)

Pond NE Basin: NE Basin



Summary for Pond SW Basin: SW Basin

Inflow Area	=	16.679 ac,	0.00% Impe	ervious,	Inflow Depth =	0.89"	for 25-y	r,24-hr	event
Inflow	=	8.28 cfs @	11.94 hrs,	Volume	= 1.237	af			
Outflow	=	4.98 cfs @	12.12 hrs,	Volume	= 1.237	af, Att	en= 40%,	Lag= 1	10.7 min
Discarded	=	4.98 cfs @	12.12 hrs,	Volume	= 1.237	af			
Primary	=	0.00 cfs @	0.00 hrs,	Volume	= 0.000	af			

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 15.03' @ 12.12 hrs Surf.Area= 129,930 sf Storage= 3,448 cf

Plug-Flow detention time= 11.5 min calculated for 1.236 af (100% of inflow) Center-of-Mass det. time= 11.5 min (921.9 - 910.4)

Volume	Inve	rt Avail.Sto	rage	Storage I	Description	
#1	15.00	D' 706,4 ²	12 cf	Custom	Stage Data (Pr	ismatic)Listed below (Recalc)
Elevatio	on S	Surf.Area	Inc.	Store	Cum.Store	
(fee	et)	(sq-ft)	(cubic	-feet)	(cubic-feet)	
15.0	00	129,810		0	0	
16.0	00	134,325	13	2,068	132,068	
17.0	00	138,901	13	6,613	268,681	
18.0	00	143,540	14	1,221	409,901	
19.0	00	148,240	14	5,890	555,791	
20.0	00	153,002	15	0,621	706,412	
Device	Routing	Invert	Outle	t Devices	i	
#1	Primary	18.00'	Asyn	nmetrical	Weir, C= 3.27	
			Offse	t (feet) 0	.00 6.00 16.00	22.00
			Heigł	nt (feet) 2	2.00 0.00 0.00	2.00
#2	Discardeo	15.00'	2.970) in/hr Ex	filtration over	Surface area
			Cond	uctivity to	Groundwater	Elevation = 14.00'
Discourd			10	10 hm 1		

Discarded OutFlow Max=9.17 cfs @ 12.12 hrs HW=15.03' (Free Discharge) **2=Exfiltration** (Controls 9.17 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=15.00' (Free Discharge)

HydroCAD® 10.00-25 s/n 05804 © 2019 HydroCAD Software Solutions LLC





Summary for Pond SWF: Southwest Forebay

Inflow Are	a =	13.035 ac,	0.00% Impe	ervious,	Inflow Depth	= 0.9	96" for	25-yı	r,24-hr ev	ent
Inflow	=	6.70 cfs @	11.91 hrs,	Volume	= 1.03	38 af				
Outflow	=	5.46 cfs @	11.99 hrs,	Volume	= 1.03	38 af,	Atten=	19%,	Lag= 4.4	min
Primary	=	5.46 cfs @	11.99 hrs,	Volume	= 1.03	38 af			-	
i minary	_	0.40 013 @	11.55 113,	Volume	- 1.0	50 ai				

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Starting Elev= 17.00' Surf.Area= 16,959 sf Storage= 30,840 cf Peak Elev= 17.13' @ 11.99 hrs Surf.Area= 17,166 sf Storage= 33,012 cf (2,172 cf above start)

Plug-Flow detention time= 512.3 min calculated for 0.330 af (32% of inflow) Center-of-Mass det. time= 9.6 min (906.0 - 896.4)

Volume	Inv	ert Ava	il.Storage	Storage	Description	
#1	15.0	20'	89,207 cf	Custom	i Stage Data (P	rismatic)Listed below (Recalc)
Elevatio (fee	on et)	Surf.Area (sq-ft)	lno (cubi	c.Store ic-feet)	Cum.Store (cubic-feet)	
15.0)0	13,915		0	0	
16.0	00	15,403		14,659	14,659	
17.0	00	16,959		16,181	30,840	
18.0	00	18,585		17,772	48,612	
19.0	00	20,280		19,433	68,045	
20.0	00	22,044	2	21,162	89,207	
Device	Routing	Ir	vert Out	let Device	S	
#1	Primary	17	7.00' Asy	mmetrica	al Weir, C= 3.27	,
			Offs	et (feet)	0.00 4.00 40.0	0 44.00
			Hei	ght (feet)	2.00 0.00 0.00	2.00
Drimary	OutFlow	1 Max-5 32	cfe @ 11	00 bre H\	N/-17 13' (Free	Discharge)

Primary OutFlow Max=5.32 cfs @ 11.99 hrs HW=17.13' (Free Discharge) —1=Asymmetrical Weir (Weir Controls 5.32 cfs @ 1.15 fps)



Pond SWF: Southwest Forebay

Summary for Pond WF: West Forebay

Inflow Area	=	31.093 ac,	1.09% Impervious,	Inflow Depth =	0.88" for 25-yr,24-hr event
Inflow	=	12.72 cfs @	12.17 hrs, Volume	= 2.292 a	af
Outflow	=	11.96 cfs @	12.24 hrs, Volume	= 2.292 a	af, Atten= 6%, Lag= 4.3 min
Primary	=	11.96 cfs @	12.24 hrs, Volume	= 2.292 a	af

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Starting Elev= 7.00' Surf.Area= 20,143 sf Storage= 35,775 cf Peak Elev= 7.22' @ 12.24 hrs Surf.Area= 20,484 sf Storage= 40,144 cf (4,369 cf above start)

Plug-Flow detention time= 267.3 min calculated for 1.469 af (64% of inflow) Center-of-Mass det. time= 9.9 min (923.0 - 913.1)

Volume	lr	nvert A	vail.Sto	rage	Storage	Description				
#1	Ę	5.00'	103,19	90 cf	Custon	n Stage Data	(Prismatic)L	_isted below	(Recalc)	
Elevatio	on et)	Surf.Are (sq-	ea ft)	Inc. (cubic	Store -feet)	Cum.Stor (cubic-fee	re <u>≥t)</u>			
5.0	00	13,9 ⁻	18		0		0			
6.0	00	18,74	14	10	5,331	16,33	31			
7.0	00	20,14	43	19	9,444	35,77	75			
8.0	00	21,73	30	20	0,937	56,71	11			
9.0	00	23,19	91	22	2,461	79,17	/2			
10.0	00	24,84	16	24	4,019	103,19) 0			
Device	Routin	g	Invert	Outle	t Device	s				
#1	Primar	у	7.00'	Spill	way, C=	3.27				
				Unse	t (leet)		00 2 00			
				Heigi	it (ieet)	3.00 0.00 0.	00 3.00			
Primary	OutFlo	w Max=1	1.88 cfs	@ 12.2	24 hrs F	HW=7.21' (Fr	ree Discharg	le)		

1=Spillway (Weir Controls 11.88 cfs @ 1.48 fps)

Pond WF: West Forebay



Summary for Link DP-1: Discharge Point 1

Inflow /	Area	=	66.671 ac,	0.51% Impervious,	Inflow Depth =	0.00" for	^{25-yr,24-hr event}
Inflow	=	=	0.00 cfs @	12.48 hrs, Volume	e= 0.000 a	af	
Primary	y =	=	0.00 cfs @	12.48 hrs, Volume	e= 0.000 a	af, Atten=	0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs



Link DP-1: Discharge Point 1

Summary for Link DP-2: Discharge Point 2

Inflow A	Area =	16.679 ac,	0.00% Impervious,	Inflow Depth = 0.0	00" for 25-yr,24-hr event
Inflow	=	0.00 cfs @	0.00 hrs, Volume	= 0.000 af	
Primary	y =	0.00 cfs @	0.00 hrs, Volume	= 0.000 af,	Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Link DP-2: Discharge Point 2


Summary for Link Link 1: Into NE Basin

Inflow /	Area	=	26.711 ac,	0.00% Imperviou	s, Inflow Depth =	0.78"	for 25-yr,24-hr event
Inflow		=	7.19 cfs @	12.37 hrs, Volur	ne= 1.739) af	
Primary	у	=	7.19 cfs @	12.37 hrs, Volur	ne= 1.739	af, A	tten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

25-yr,24-hr Inflow Imported from Indian River Landfil (ABCD)~Link Link 1.hce



Link Link 1: Into NE Basin